

Integrated Multiscale Hazard Scenarios

**Intermediate-term
medium-range
earthquake
predictions (CN, M8S)**

**Pattern recognition
of earthquake prone
areas (nodes)**

**Restrained area
for expected
sources + time**

Multiscale
structural models

Seismic sources
characterization

**Multiscale
Ground motion
scenarios**

**Space &
time
info for
Seismic Risk**

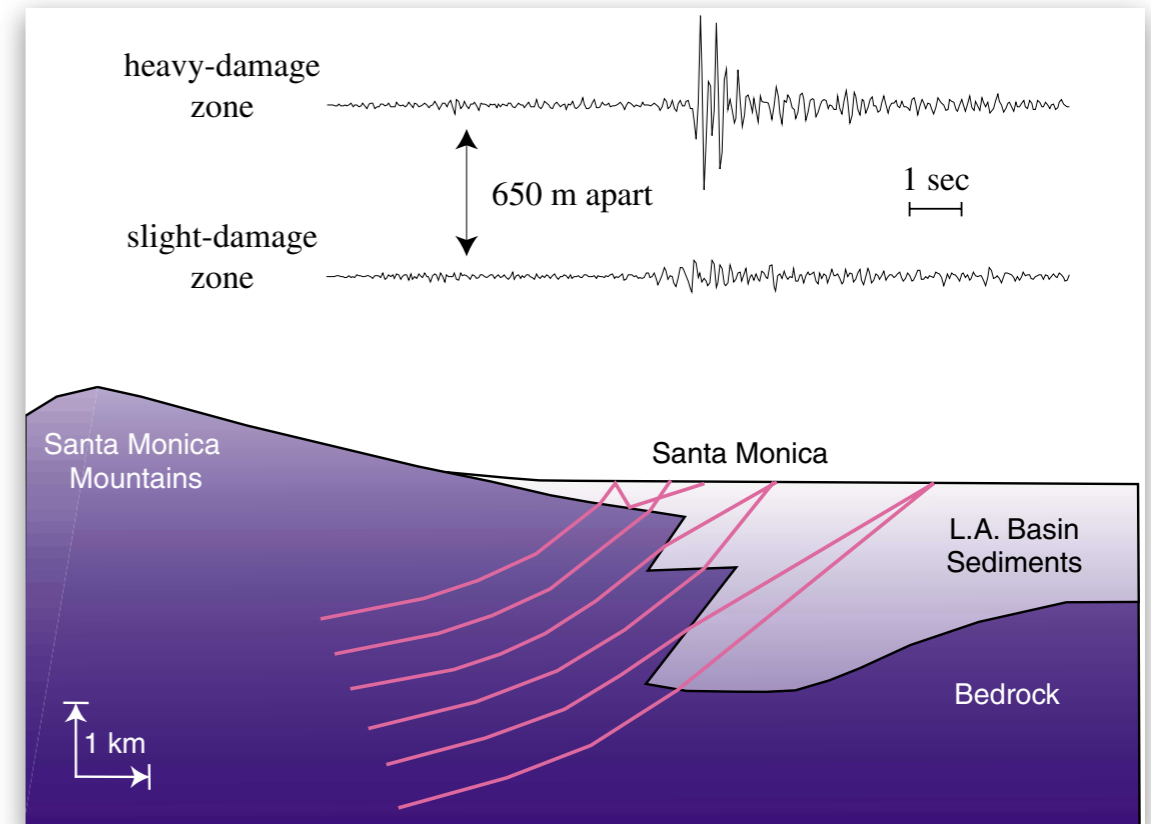
**Seismic
Input
for engineering
analysis**

SURFACE TOPOGRAPHY EFFECTS

(convexity) sensitivity to:

- a) type of wavefield
- b) angle of incidence
- c) shape and sharpness

GROUNDSHAKING SITE EFFECTS



SOFT SURFACE LAYERING

a) 1-D: trapping of waves for impedance contrast; vertical resonances

$$f_n = [(2n+1)\beta]/4H; A \approx (\rho_2 v_2)/(\rho_1 v_1)$$

b) 2/3-D 3-D: complex energy focusing; diffraction effects;
basin edge waves

WEAK (AND STRONG) MOTION

- a) S/B spectral ratio (Borcherdt, 1970)
- b) generalized inversion scheme (Andrews, 1986)
- c) coda waves analysis (Margheriti et al., 1994)
- d) parametrized source and path inversion (Boatwright et al., 1991)
- e) H/V spectral ratio (receiver function) (Lermo et al., 1993)

EMPIRICAL TECHNIQUES FOR SITE EFFECT ESTIMATION

$$R_{ij}(\omega) = E_i(\omega) \cdot P_{ij}(\omega) \cdot S_j(\omega)$$

MICROTREMORS

- a) peak frequencies examination
- b) S/B spectral ratio
- c) H/V spectral ratio (Nagoshi, 1971; Nakamura, 1989)
- d) array analysis (Malagnini et al., 1993)

Site effects and SHA

- In SHA the site effect should be defined as the **average behavior**, relative to other sites, given all potentially damaging earthquakes
- This produces an **intrinsic variability** with respect to different earthquake locations, that cannot exceed the difference between sites
- Site characterization:
 - which velocity?
 - use of basin depth effect?

Site effects and Italian code

Categorie di sottosuolo

Ai fini della definizione dell'azione sismica di progetto, si rende necessario valutare l'effetto della risposta sismica locale mediante specifiche analisi, come indicato nel § 7.11.3. In assenza di tali analisi, per la definizione dell'azione sismica si può fare riferimento a un approccio semplificato, che si basa sull'individuazione di categorie di sottosuolo di riferimento (Tab. 3.2.II e 3.2.III).

Tabella 3.2.II – *Categorie di sottosuolo*

Categoria	Descrizione
A	<i>Ammassi rocciosi affioranti o terreni molto rigidi caratterizzati da valori di $V_{s,30}$ superiori a 800 m/s, eventualmente comprendenti in superficie uno strato di alterazione, con spessore massimo pari a 3 m.</i>
B	<i>Rocce tenere e depositi di terreni a grana grossa molto addensati o terreni a grana fina molto consistenti con spessori superiori a 30 m, caratterizzati da un graduale miglioramento delle proprietà meccaniche con la profondità e da valori di $V_{s,30}$ compresi tra 360 m/s e 800 m/s (ovvero $N_{SPT,30} > 50$ nei terreni a grana grossa e $c_{u,30} > 250$ kPa nei terreni a grana fina).</i>
C	<i>Depositi di terreni a grana grossa mediamente addensati o terreni a grana fina mediamente consistenti con spessori superiori a 30 m, caratterizzati da un graduale miglioramento delle proprietà meccaniche con la profondità e da valori di $V_{s,30}$ compresi tra 180 m/s e 360 m/s (ovvero $15 < N_{SPT,30} < 50$ nei terreni a grana grossa e $70 < c_{u,30} < 250$ kPa nei terreni a grana fina).</i>
D	<i>Depositi di terreni a grana grossa scarsamente addensati o di terreni a grana fina scarsamente consistenti, con spessori superiori a 30 m, caratterizzati da un graduale miglioramento delle proprietà meccaniche con la profondità e da valori di $V_{s,30}$ inferiori a 180 m/s (ovvero $N_{SPT,30} < 15$ nei terreni a grana grossa e $c_{u,30} < 70$ kPa nei terreni a grana fina).</i>
E	<i>Terreni dei sottosuoli di tipo C o D per spessore non superiore a 20 m, posti sul substrato di riferimento (con $V_s > 800$ m/s).</i>

La velocità equivalente delle onde di taglio $V_{s,30}$ è definita dall'espressione

$$V_{s,30} = \frac{30}{\sum_{i=1,N} \frac{h_i}{V_{s,i}}} \text{ [m/s]}.$$

Site effects and Italian code

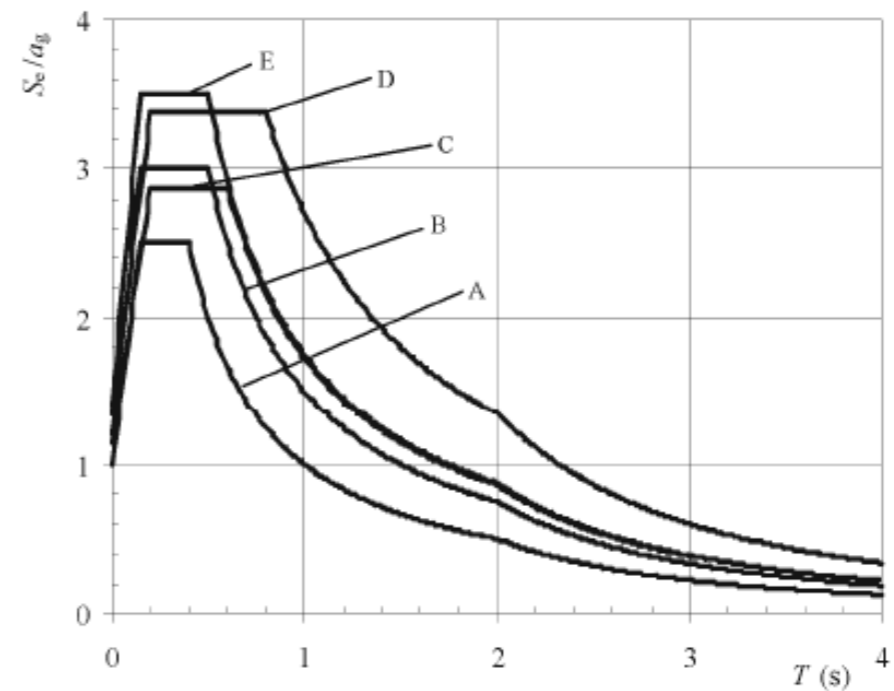
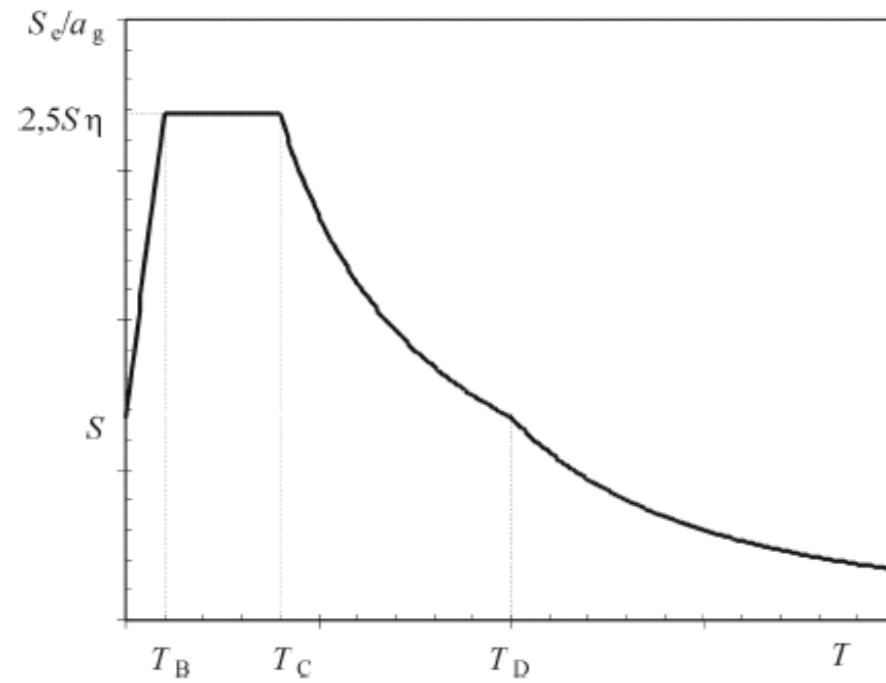
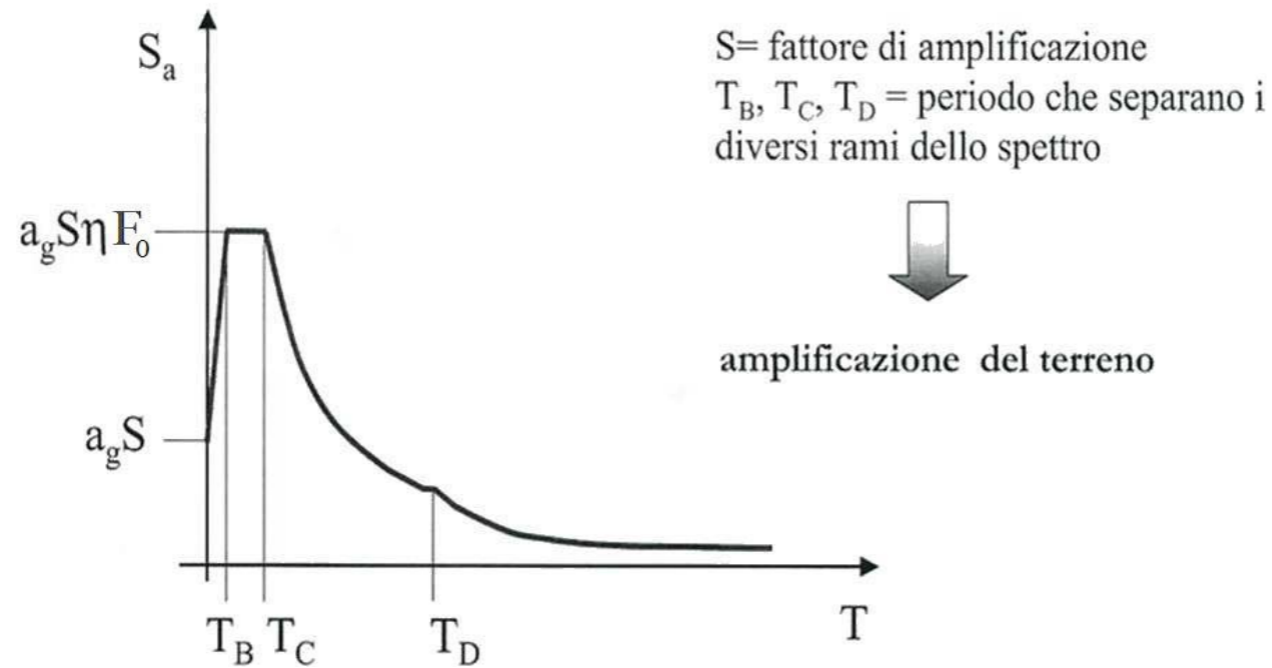
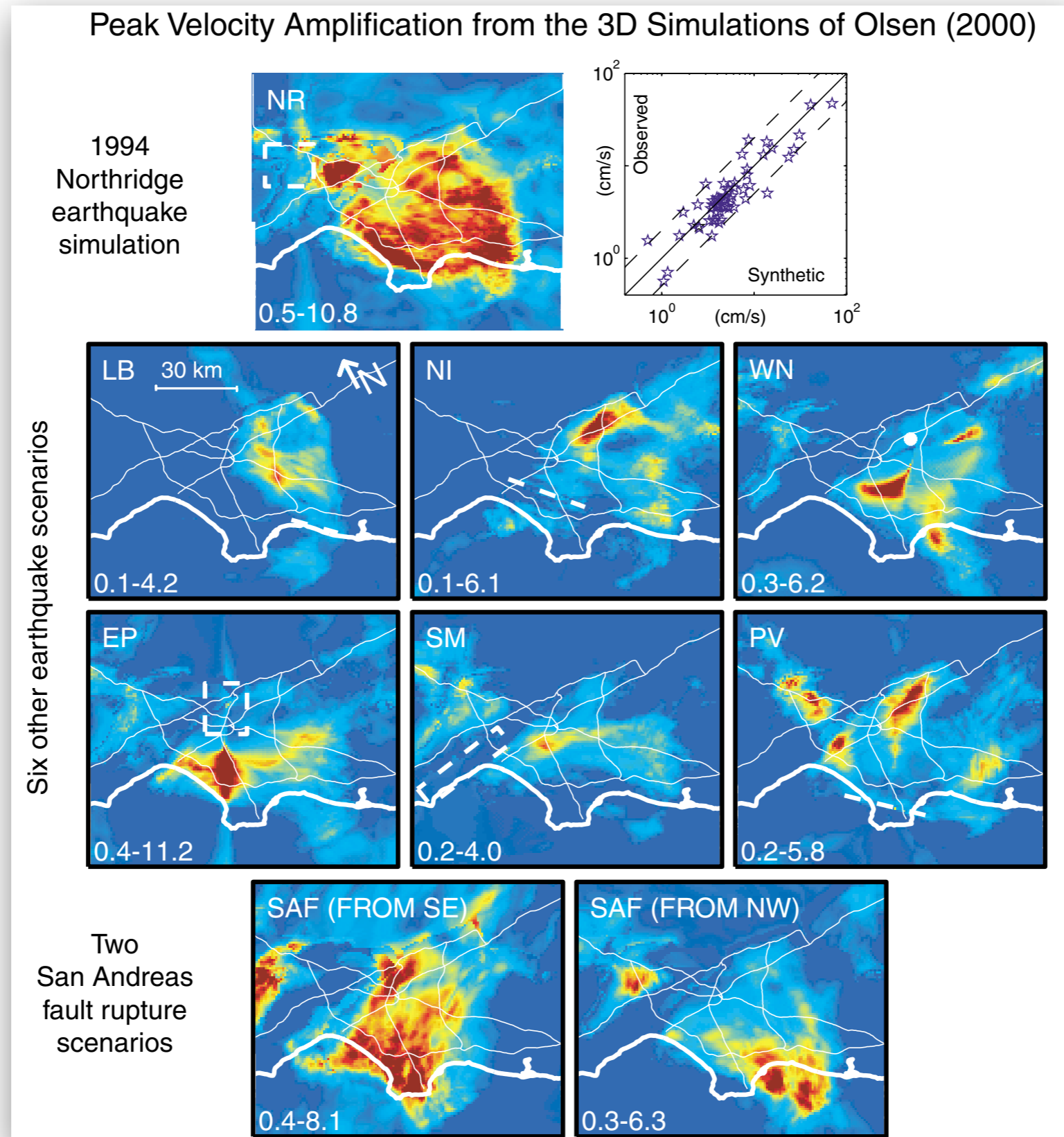


Figure 3.1: Shape of the elastic response spectrum

Amplification patterns...

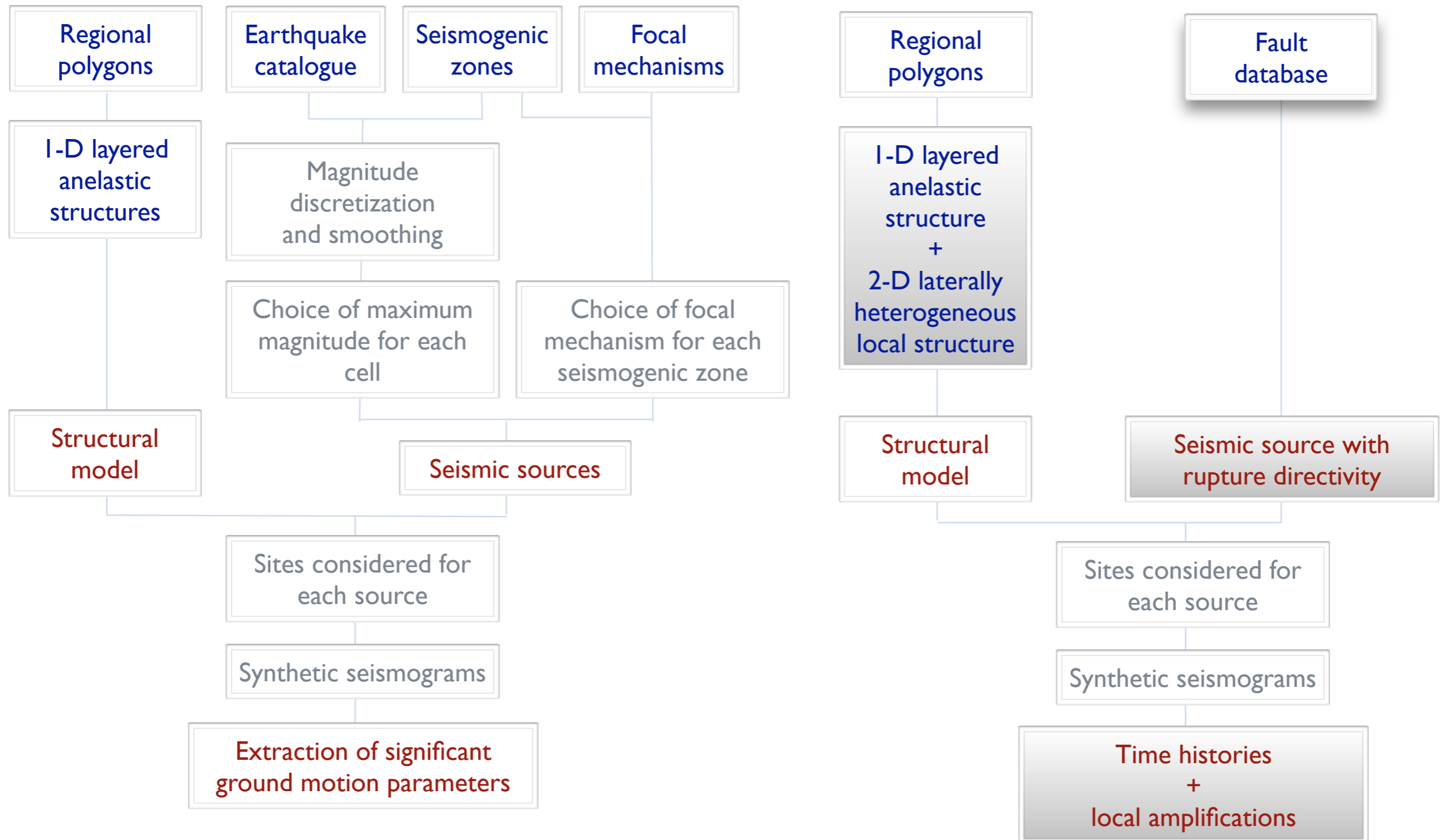
...may vary greatly among the earthquake scenarios, considering different source locations (and rupture ...)

SCEC
Phase 3
Report



Regional & Local scale

→ Local scale (2D)



Introduction - Local scale

- Synthetic seismograms along selected profiles
- Laterally heterogeneous structural models
- Detailed source models
- Cutoff frequency up to 10 Hz
- Time series, amplification maps

Introduction - Methodology

- Regional scale: modal summation
- Local scale: hybrid methodology
(modal summation + finite differences)

Methodology - Modal summation

- Displacement generated by a double-couple in layered half-space (Panza, 1985, Florsch et al 1991)

$$\begin{aligned}
 u_y^L(x, z, \omega) &= \sum_{m=1}^{\infty} \frac{e^{-i3\pi/4}}{\sqrt{8\pi\omega}} \frac{e^{-ik_m x - \omega x C_{2m}}}{\sqrt{x}} \frac{\left(\chi_m^L(h_s, \omega)\right)}{\sqrt{c_m v_m I_m}} \frac{\left(F_y(z, \omega)\right)}{\sqrt{v_m I_m}} \\
 u_x^R(x, z, \omega) &= \sum_{m=1}^{\infty} \frac{e^{-i3\pi/4}}{\sqrt{8\pi\omega}} \frac{e^{-ik_m x - \omega x C_{2m}}}{\sqrt{x}} \frac{\left(\chi_m^R(h_s, \omega)\right)}{\sqrt{c_m v_m I_m}} \frac{\left(F_x(z, \omega)\right)}{\sqrt{v_m I_m}} \\
 u_z^R(x, z, \omega) &= \sum_{m=1}^{\infty} \frac{e^{-i\pi/4}}{\sqrt{8\pi\omega}} \frac{e^{-ik_m x - \omega x C_{2m}}}{\sqrt{x}} \frac{\left(\chi_m^R(h_s, \omega)\right)}{\sqrt{c_m v_m I_m}} \frac{\left(F_z(z, \omega)\right)}{\sqrt{v_m I_m}}
 \end{aligned}$$

■ source

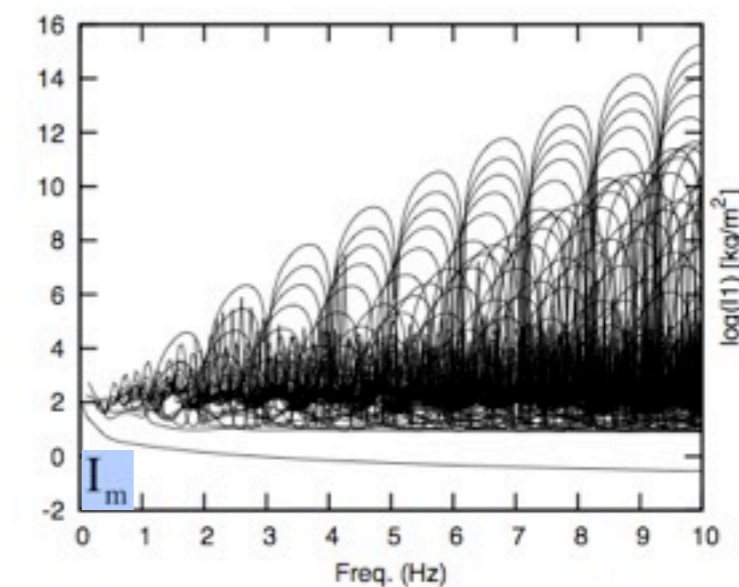
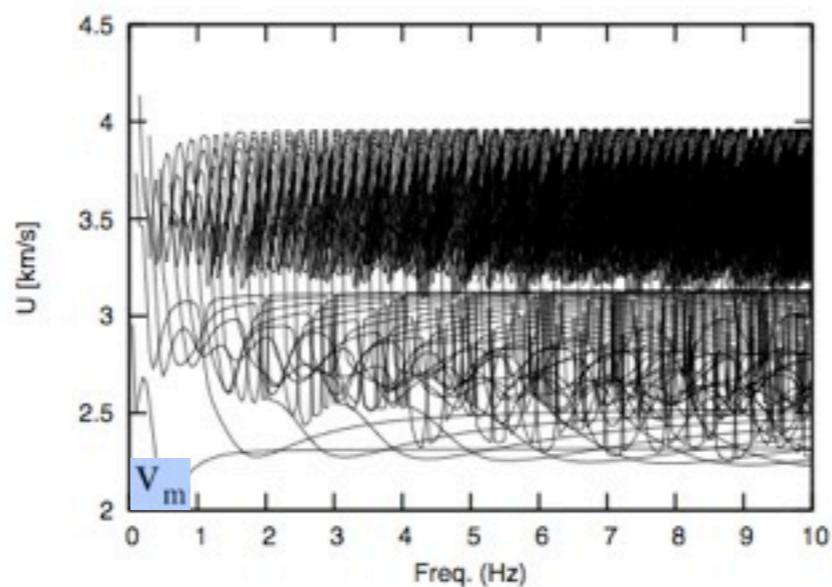
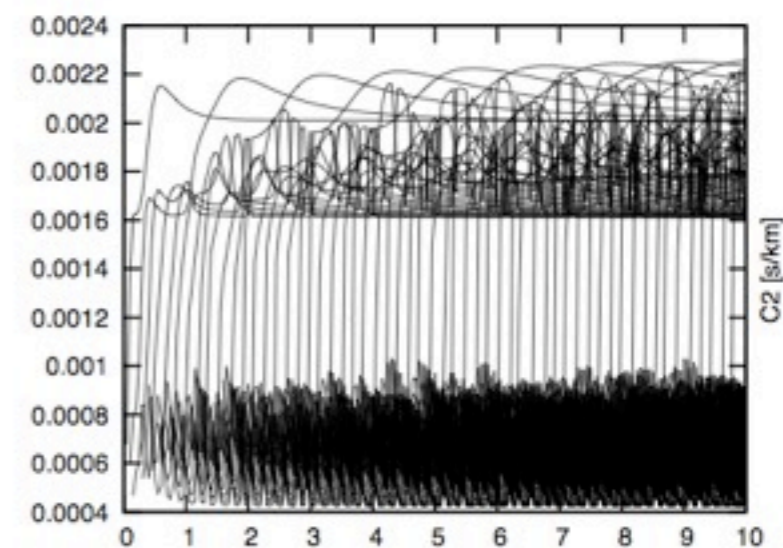
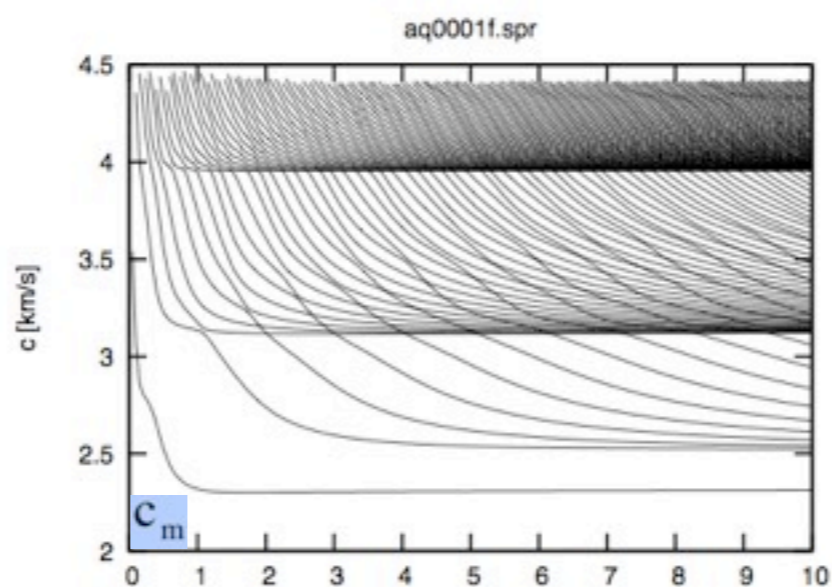
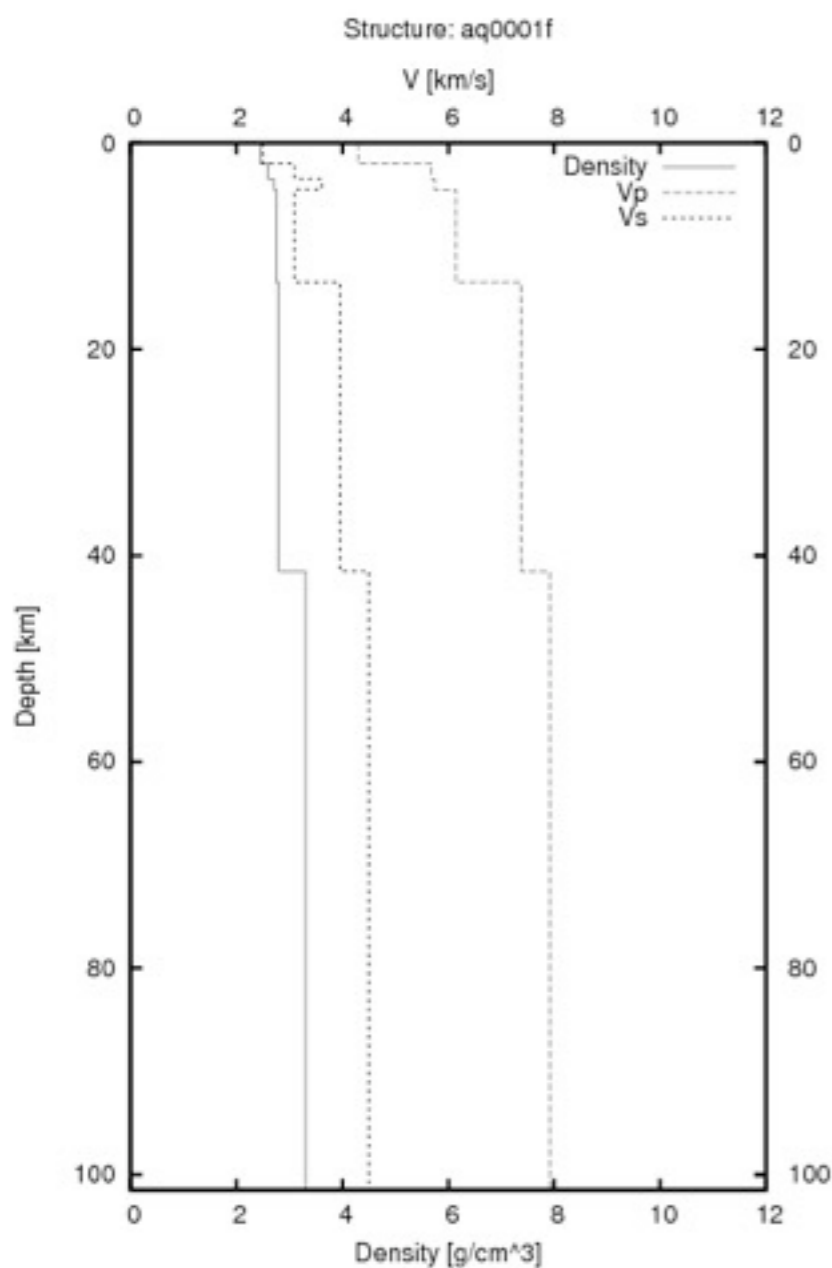
■ propagation

■ site

Methodology - Modal summation

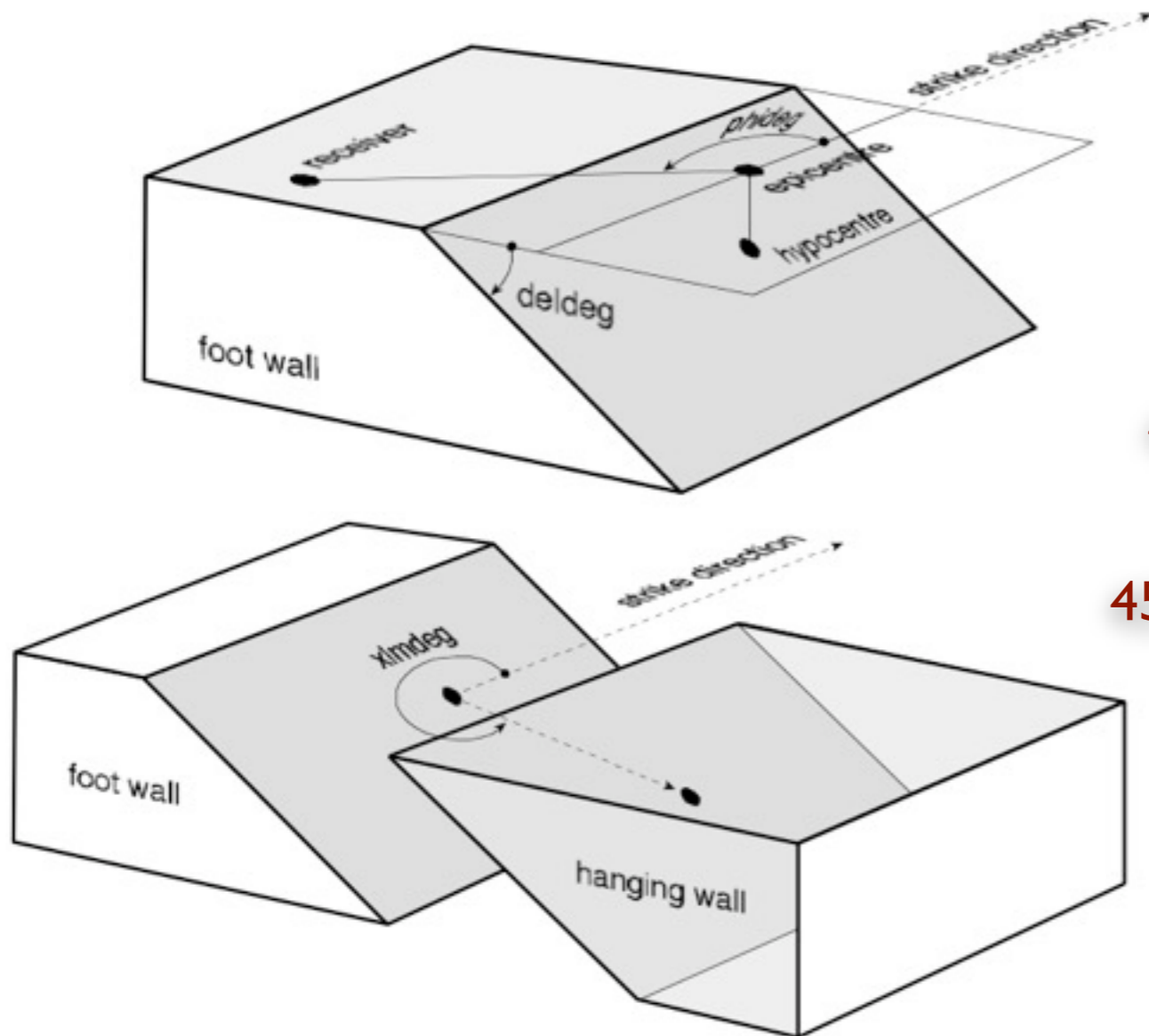
Examples of structural quantities

$$\sqrt{c_m v_m I_m} \quad \sqrt{v_m I_m}$$



Methodology - Modal summation

● Source definition and radiation pattern



vertical strike-slip

45° dipping strike-slip

45° dipping oblique slip

45° dip-slip (thrust)

45° dip-slip (normal)

vertical dip-slip

Love Rayleigh



8



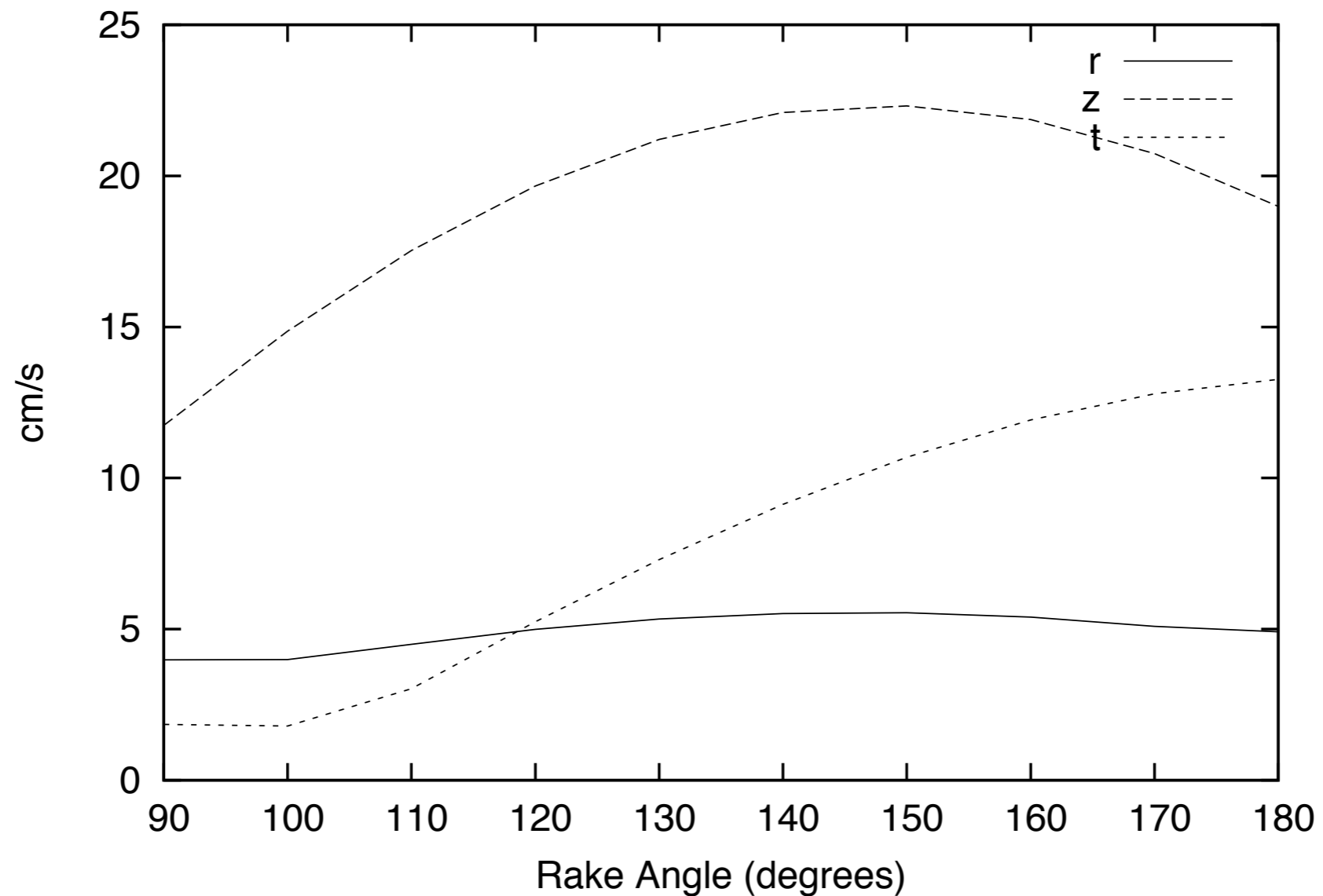
$$\left(\chi_m^L(h_s, \omega) \right)$$

$$\left(\chi_m^R(h_s, \omega) \right)$$

Methodology - Modal summation

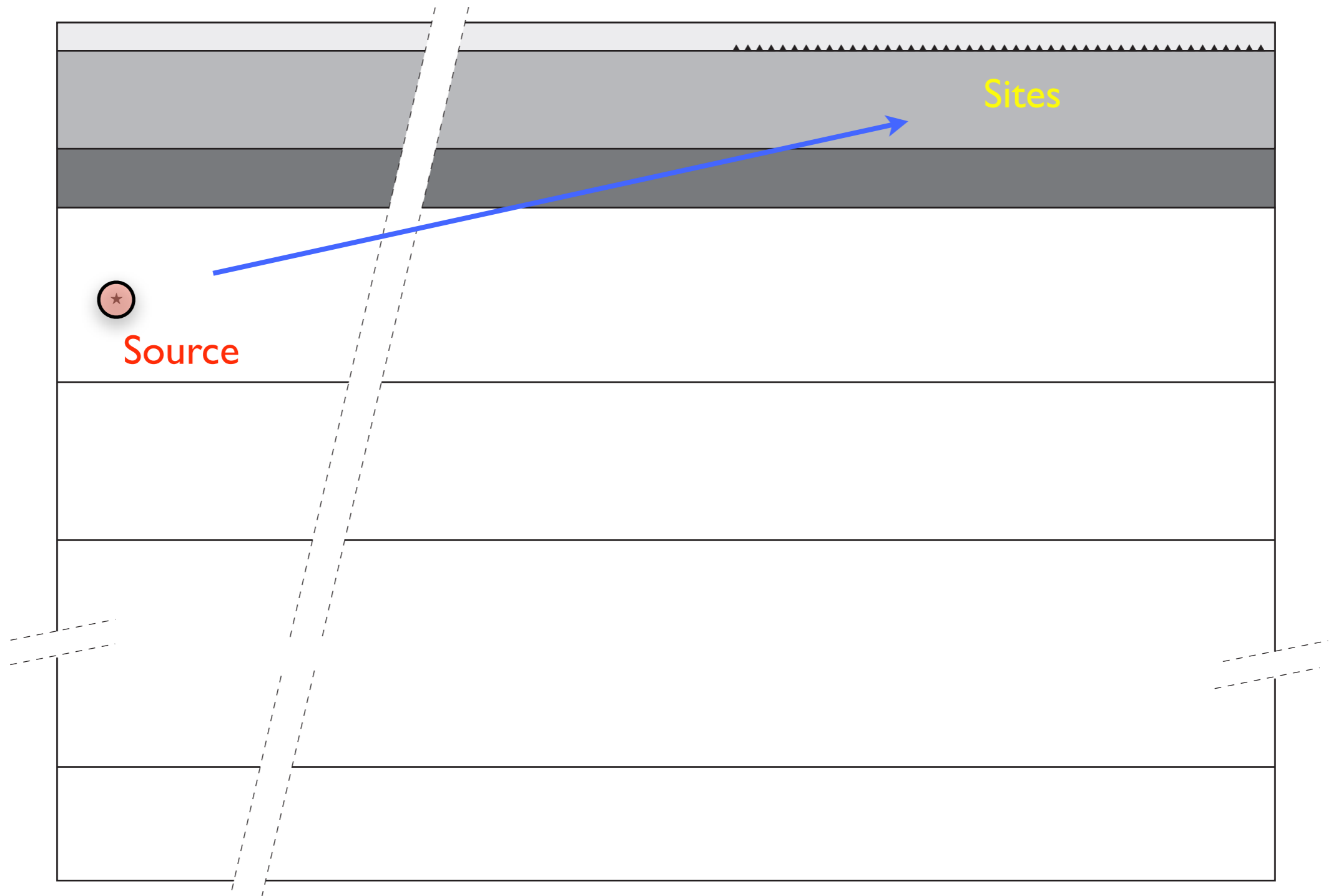
● ID parametric tests: rake variation

(s1f1) sre=168.00 dip=30.0 sde= 7.000 edi= 15.000 rde= 0.000
mod= 0- 0 int= 1 mag=6.5



Methodology - Modal summation (regional scale)

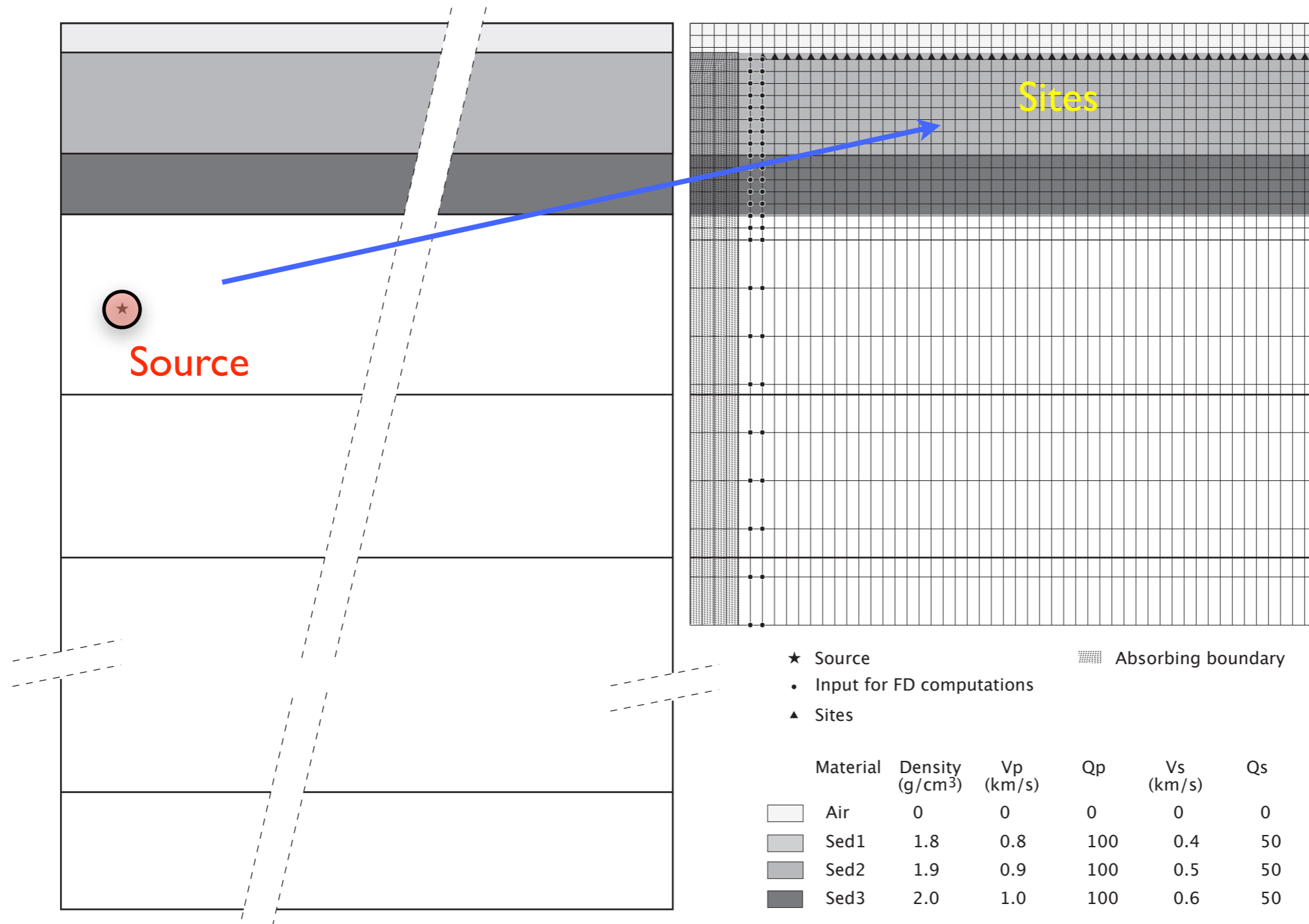
Modal summation



Methodology - Hybrid technique

Modal summation

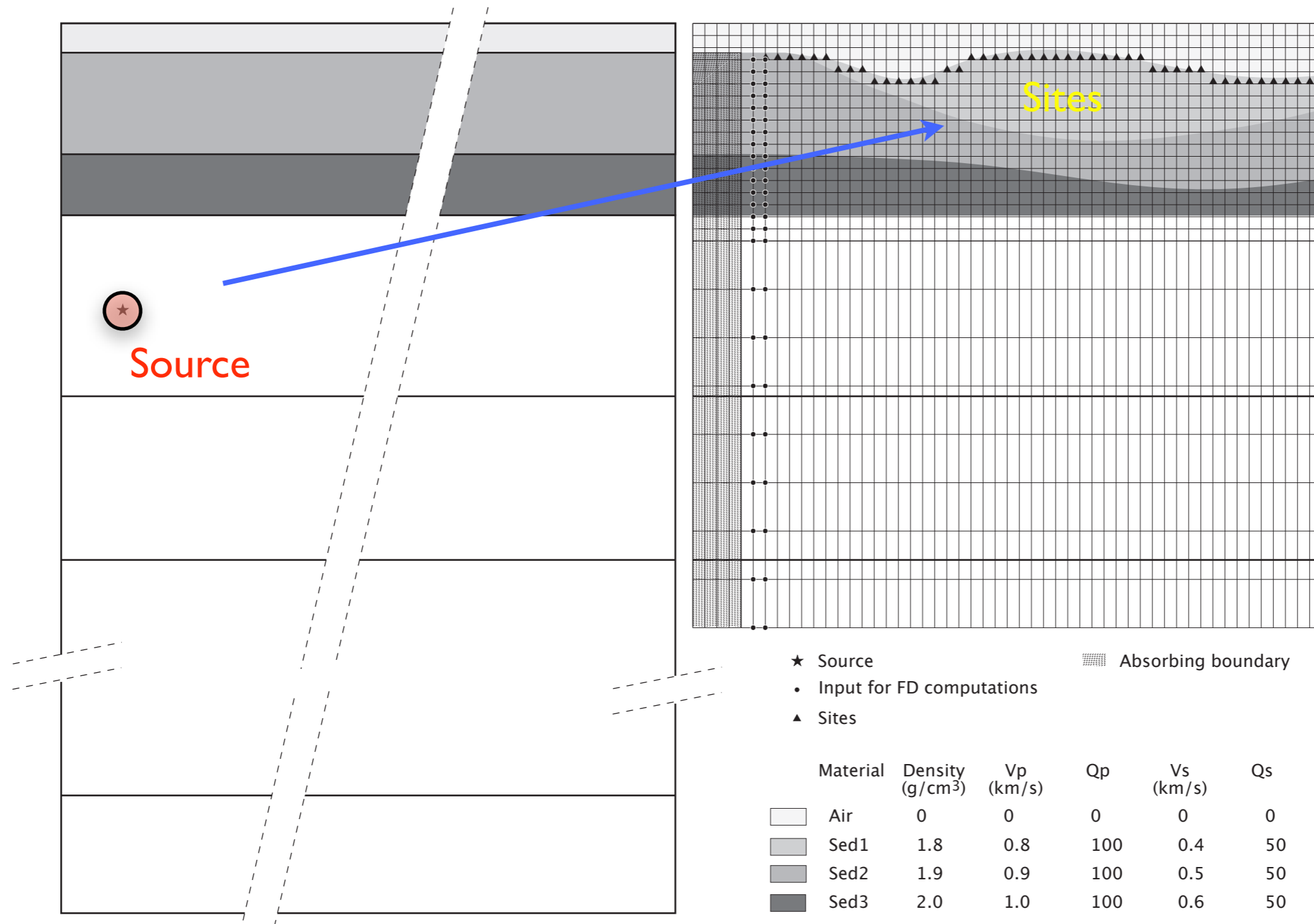
Finite Differences



Methodology - Hybrid technique (local scale)

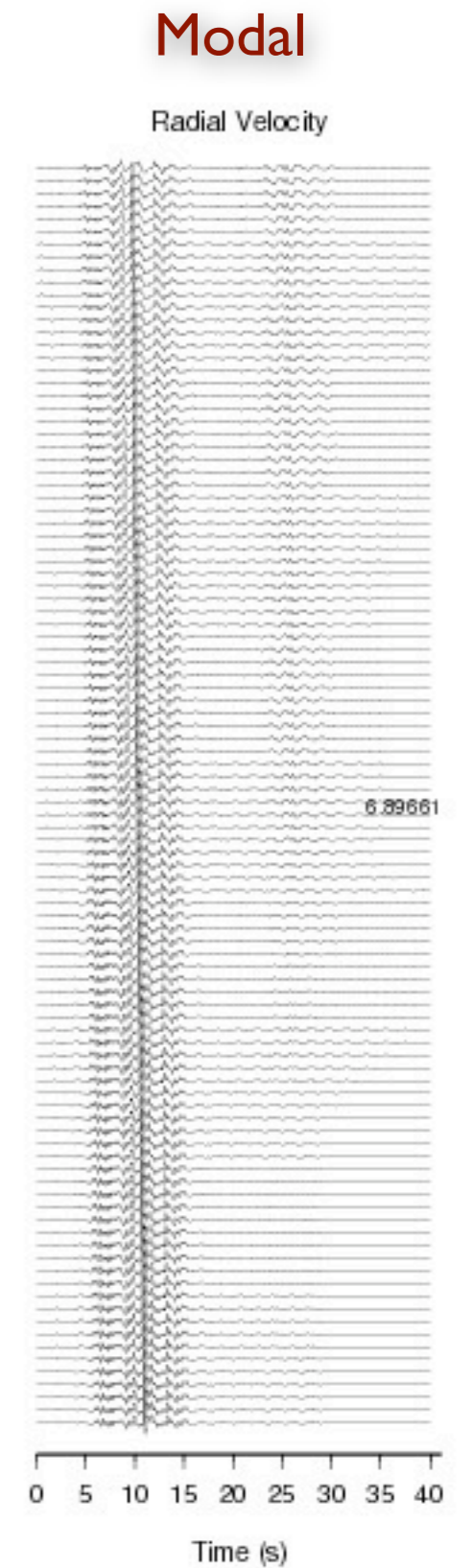
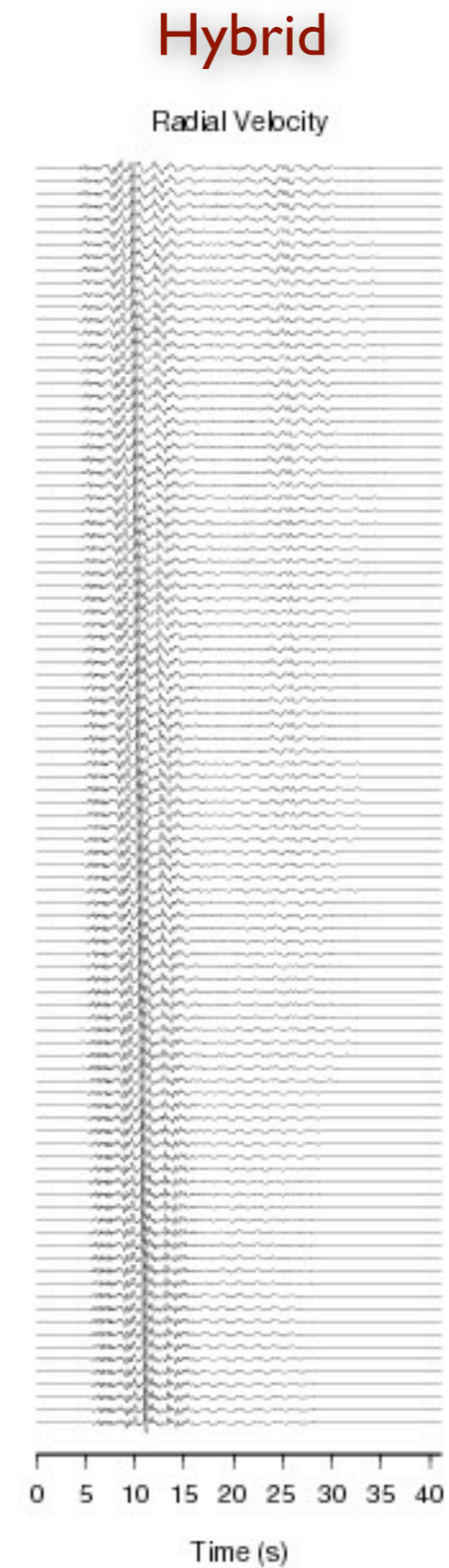
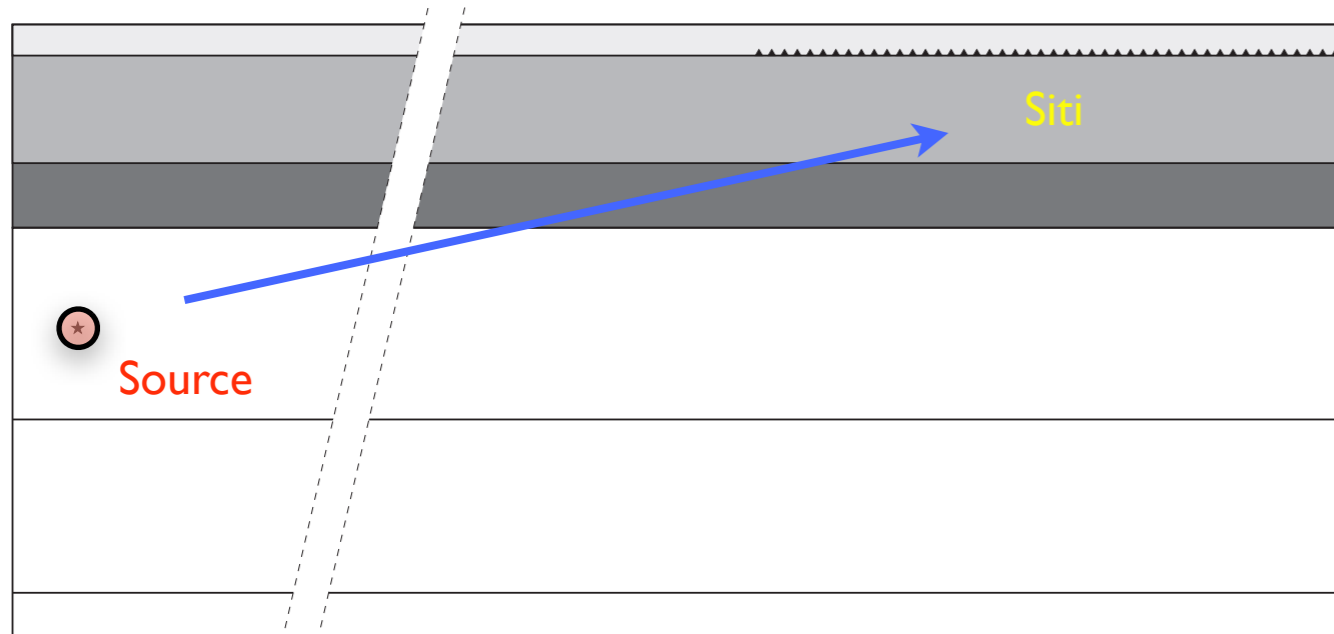
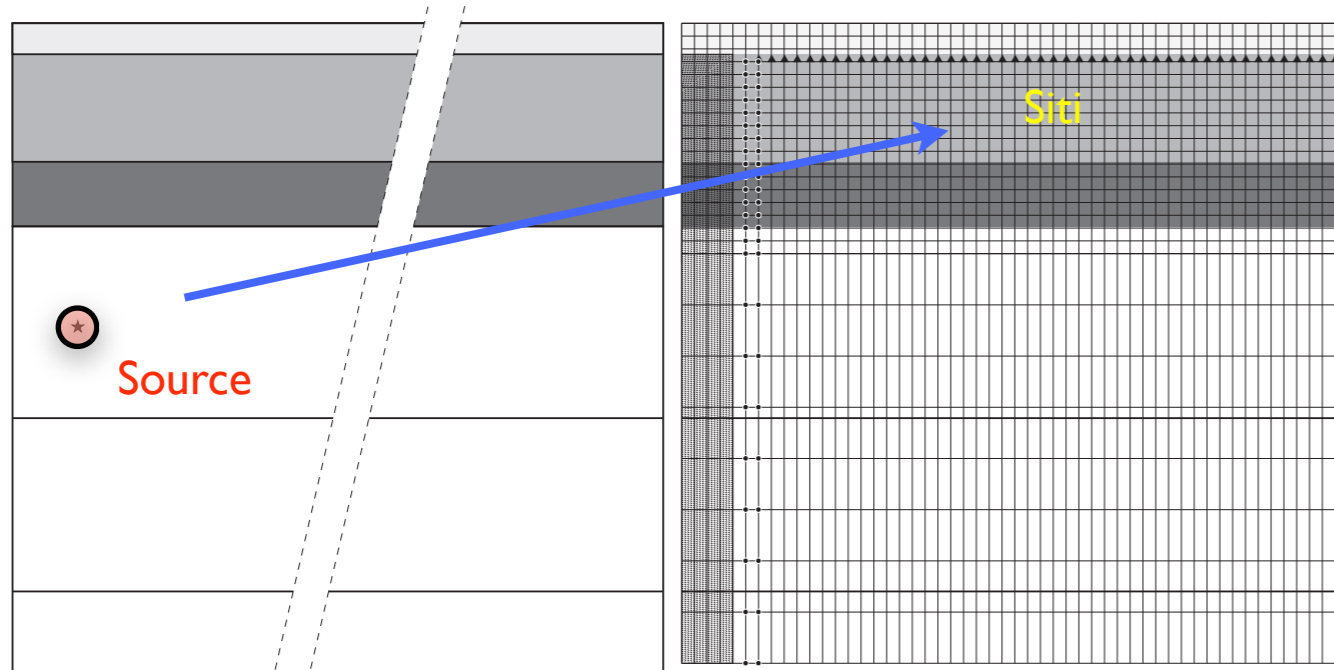
Modal summation

Finite Differences

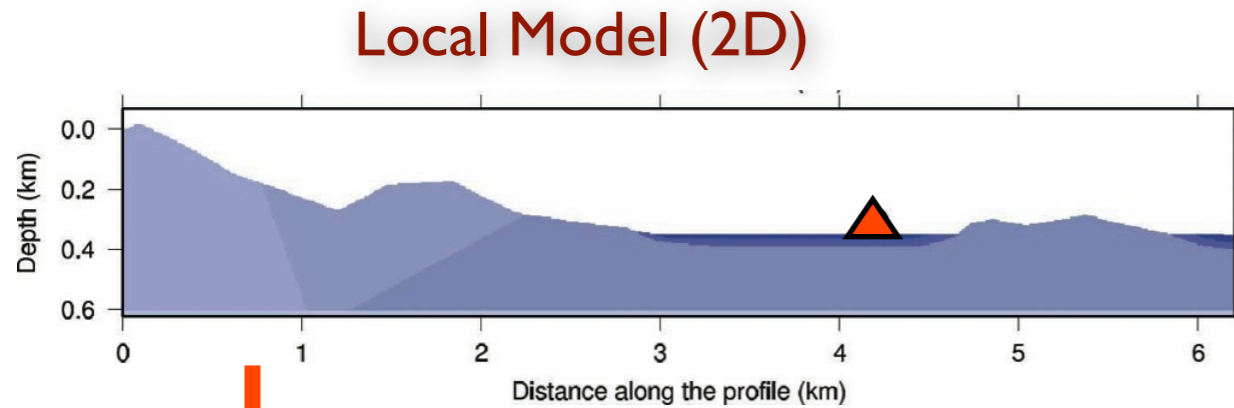


Methodology - Hybrid method

Quality test



Local Scale - Response Spectra Ratio



2D seismogram

2D response spectra

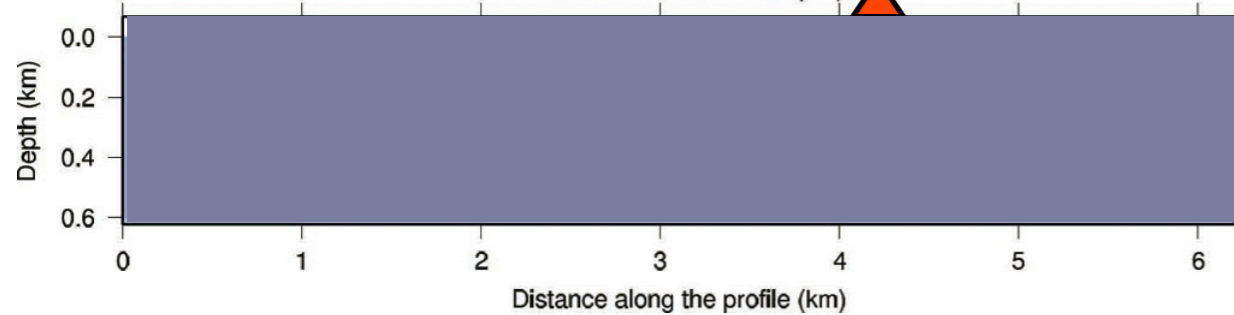
RSR
2D/1D

1D seismogram

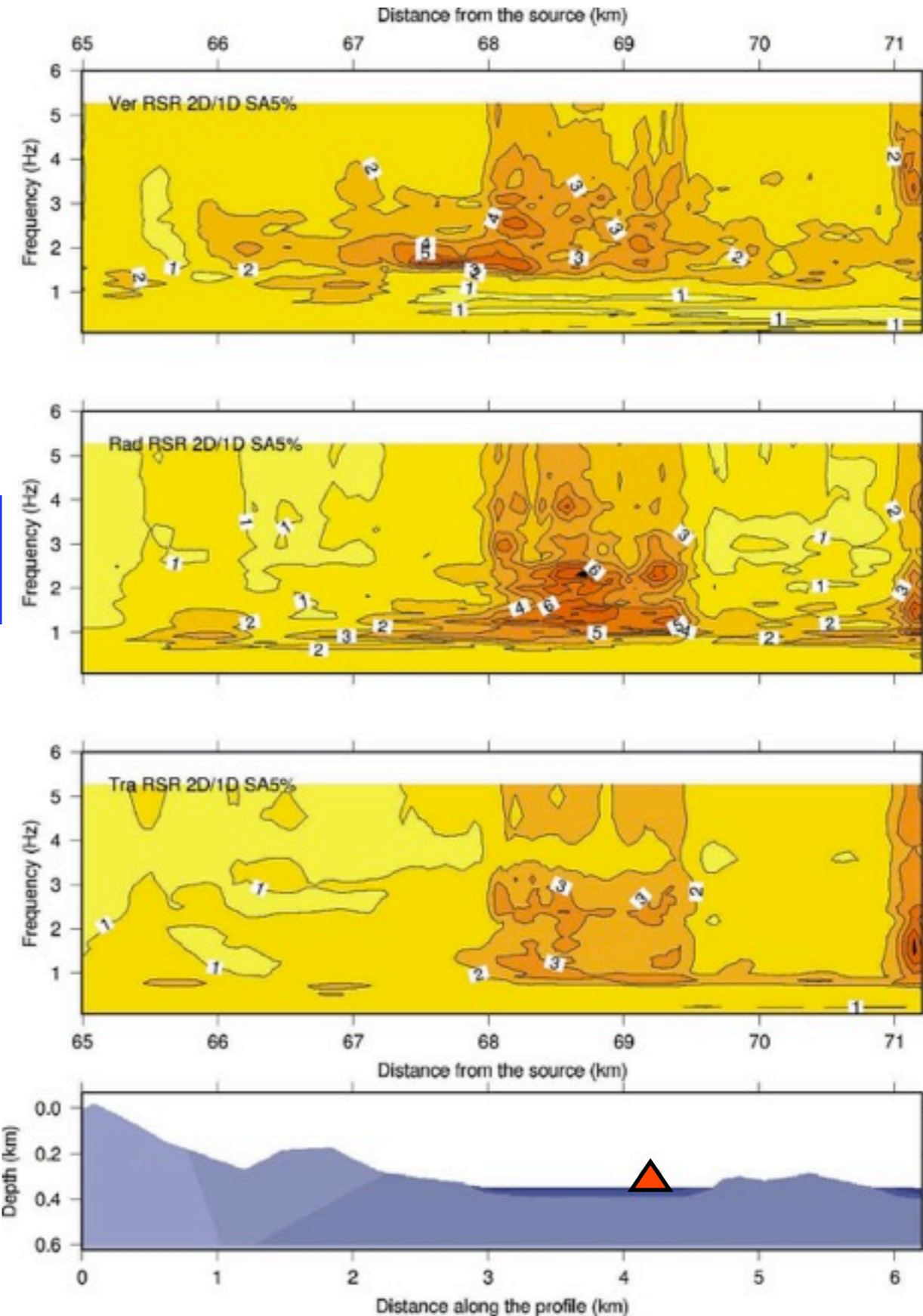
1D response spectra



Bedrock (1D)



Site effects in Trieste city centre may cause a significant amplification (up to 5 times at engineering relevant frequencies) of the seismic signal at bedrock, hence intensity may reach IX (MCS) or VIII (MSK).



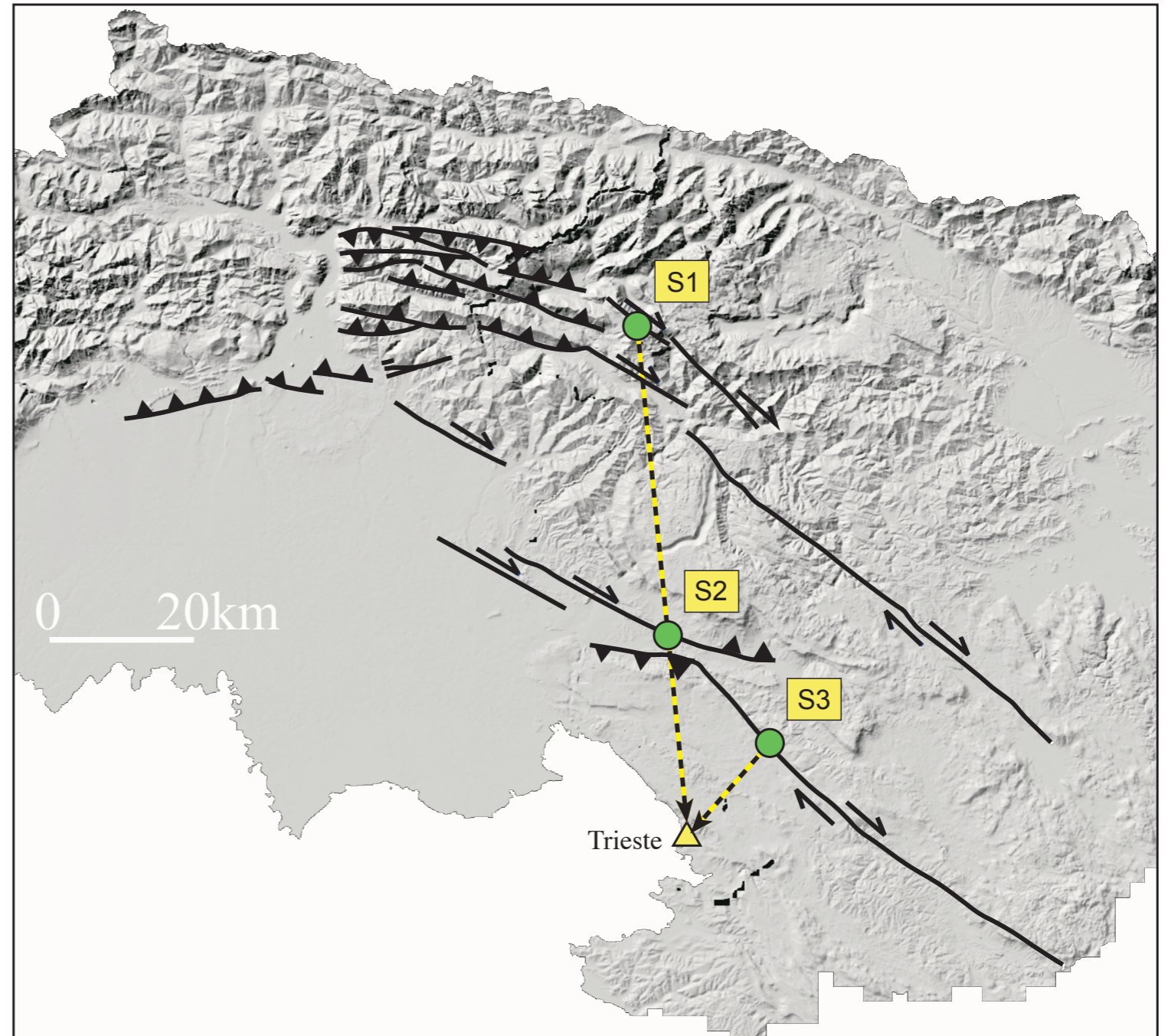
Local Scale - Choice of Scenario Earthquakes

- Regional zonation
- Morphostructural analysis
- Active faults
- Earthquake prone areas

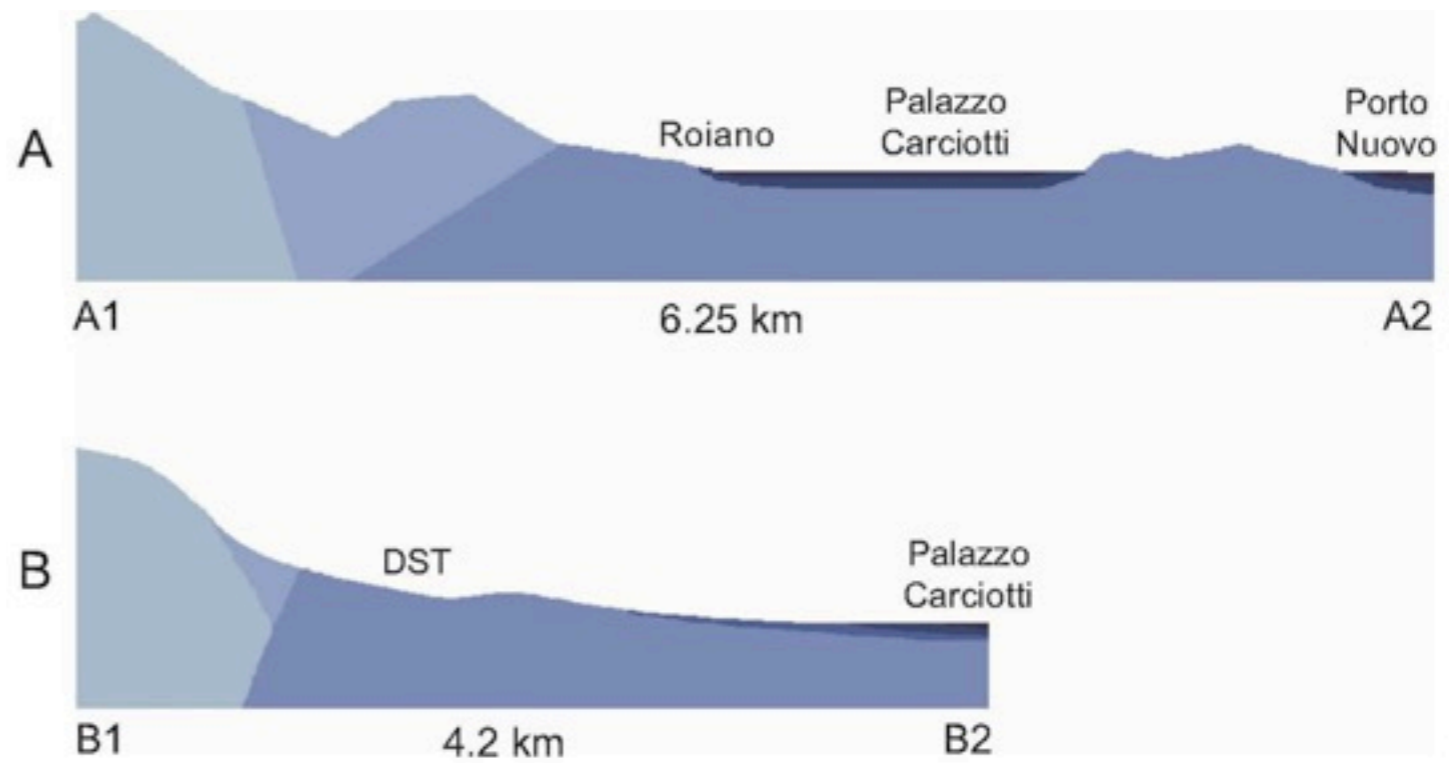
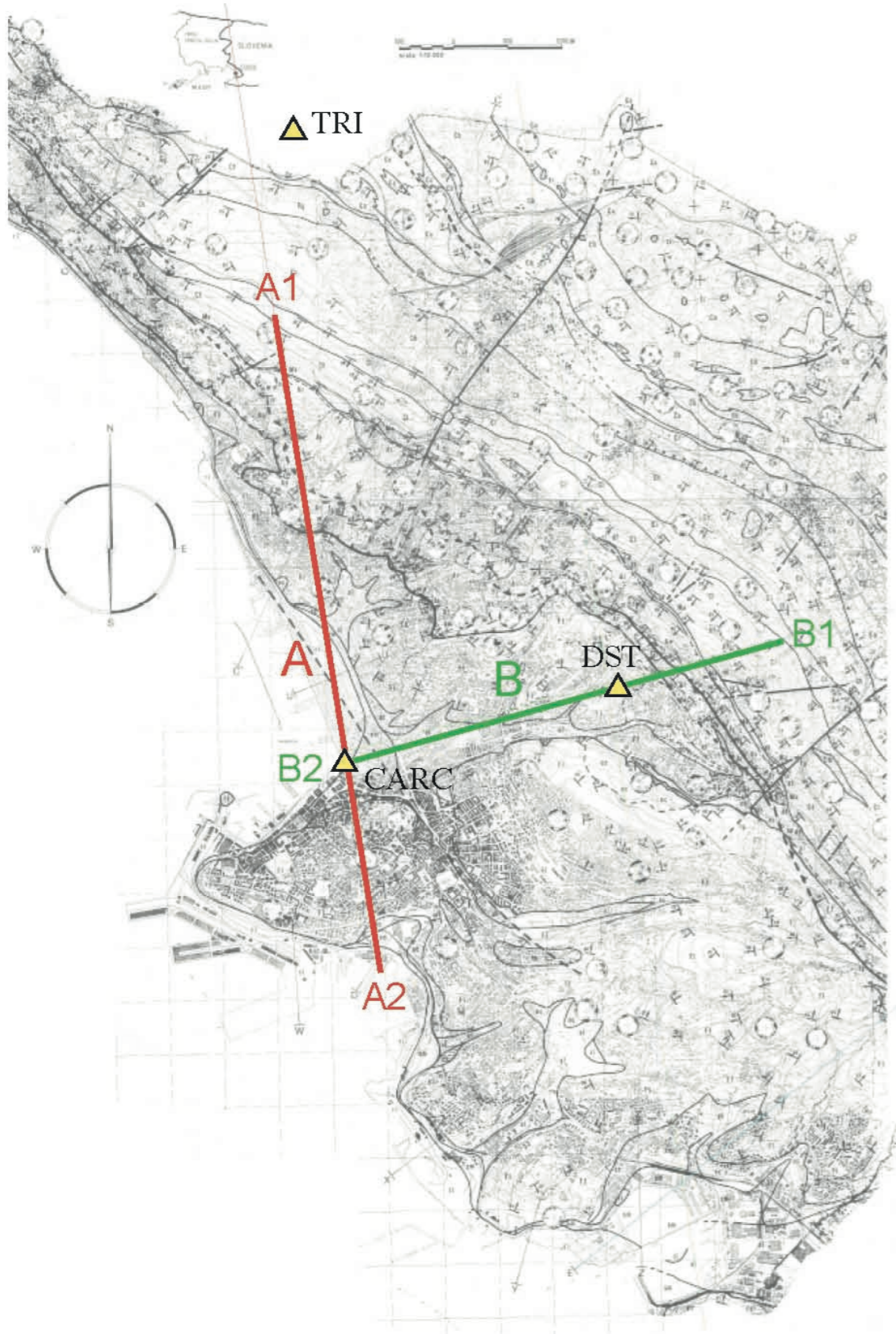
S1 in the Bovec zone
(65 km from Trieste)

S2 East of Gorizia
(30 km from Trieste)

S3 at 17 km from Trieste



Local Scale - Choice of Profiles



Litotipo	Densità (g/cm ³)	Vp (km/s)	Vs (km/s)	Qp	Qs
Riporti	1.8	0.4	0.2	30	15
Sed. Marini	1.9	0.8	0.4	40	20
Alluvioni	1.95	1.0	0.5	40	20
Flysch	2.0	1.8	1.0	100	50
Marne	2.0	1.9	1.1	200	100
Arenarie	2.1	2.0	1.2	200	100
Calcari	2.3	2.5	1.4	200	100

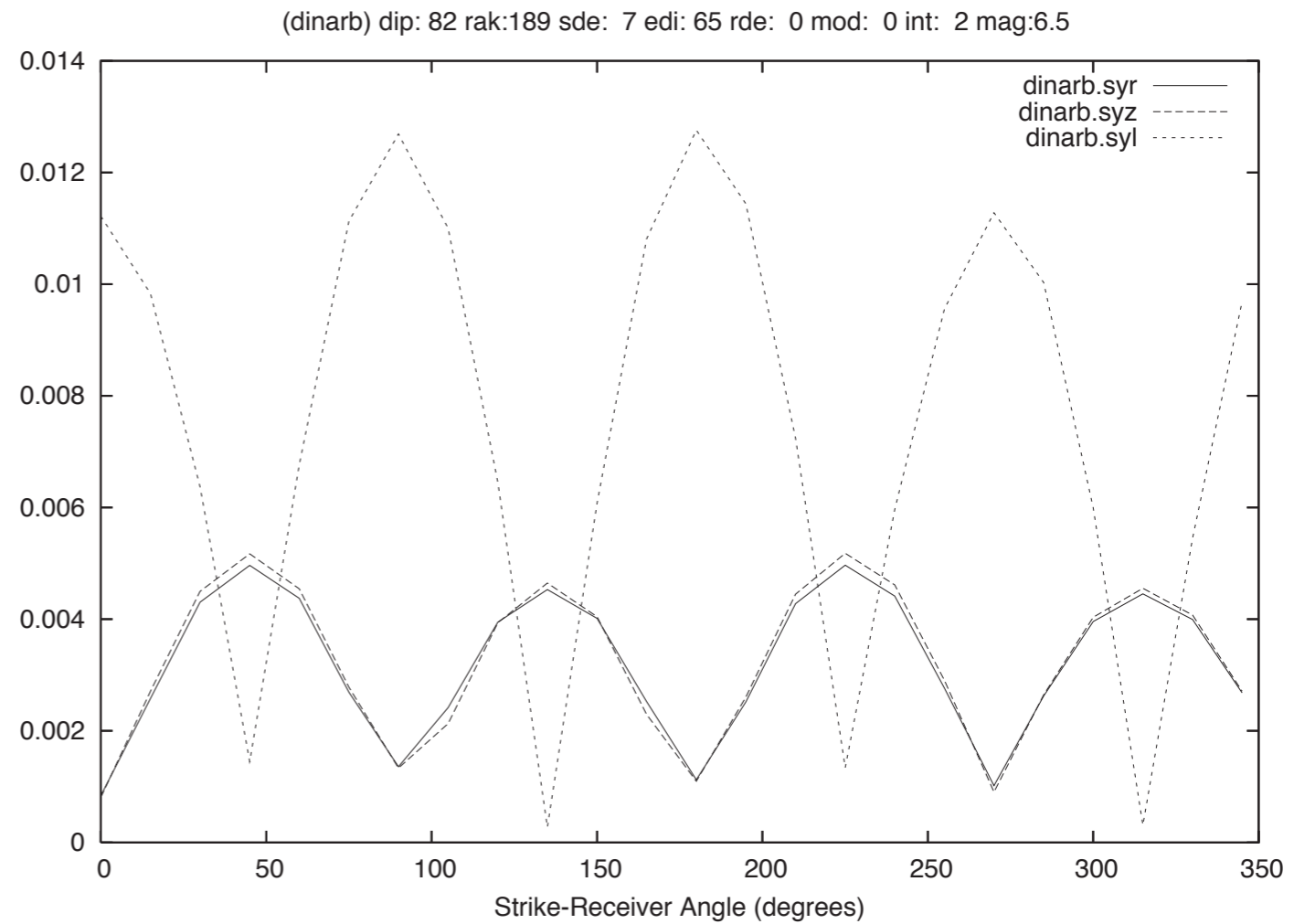
Local Scale - Preliminary Parametric Test

● Radiation Pattern

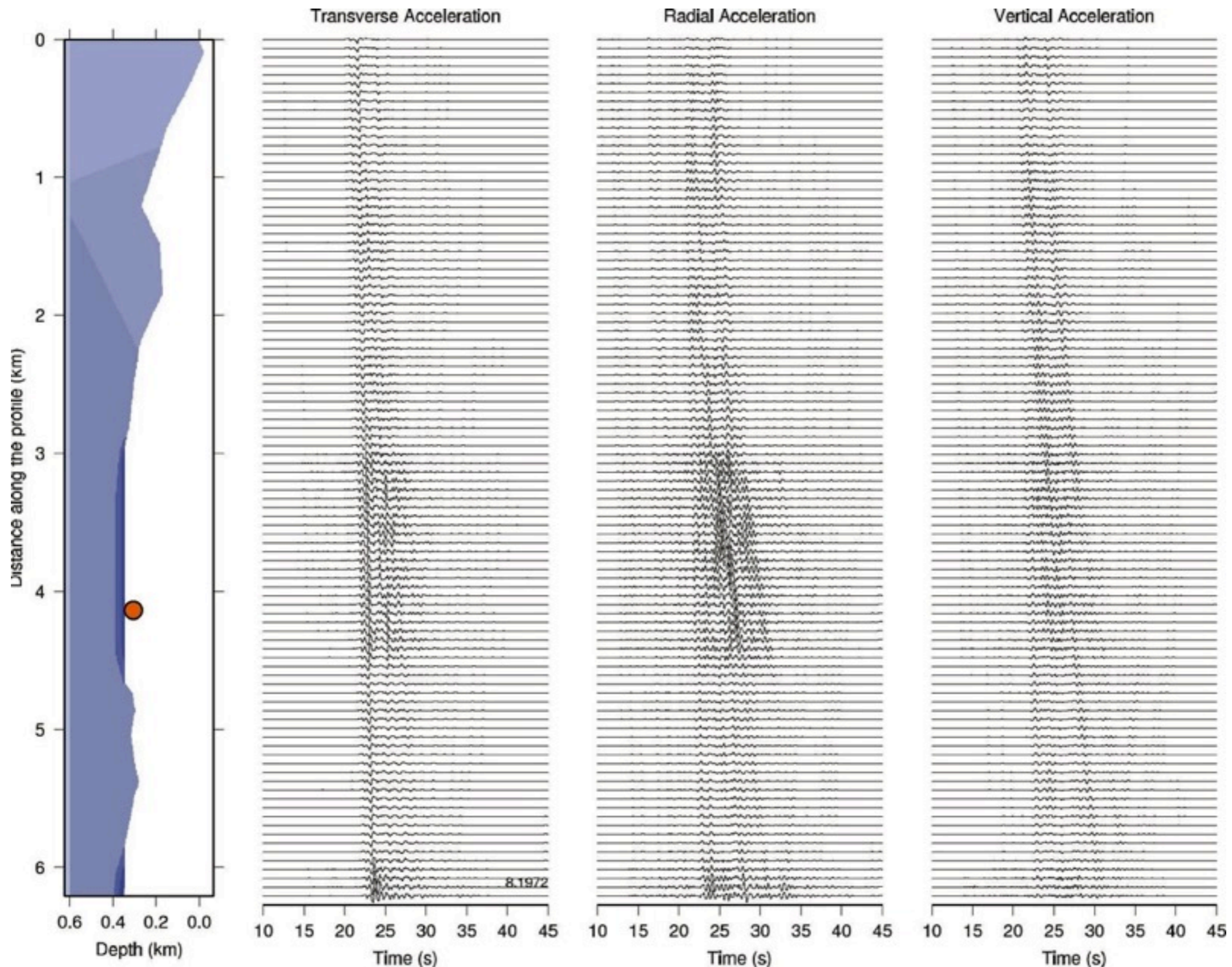
● Source Depth

● Epicentral Distance

●

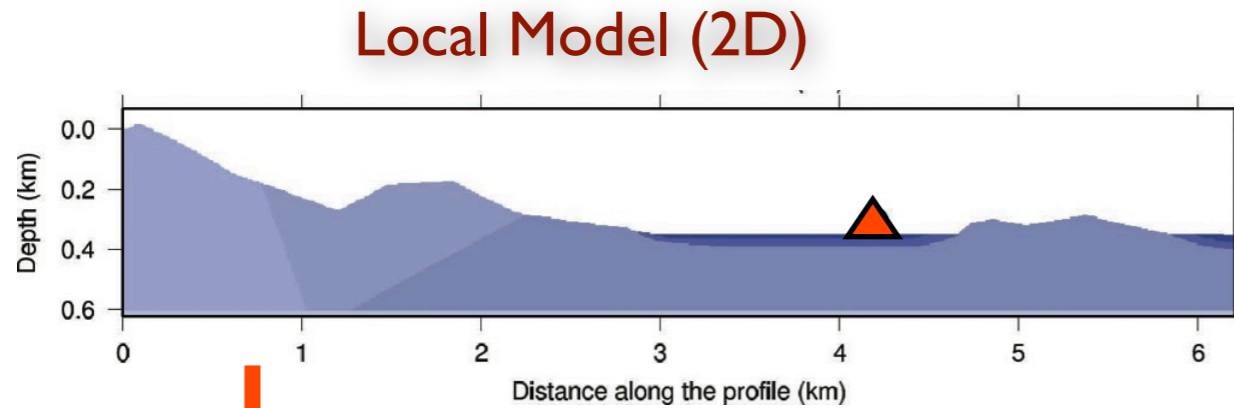


Local Scale - Synthetic Seismograms



Profile I - Bedrock "B" - Dist. 17 km - M=6.0

Local Scale - Response Spectra Ratio



2D seismogram

2D response spectra

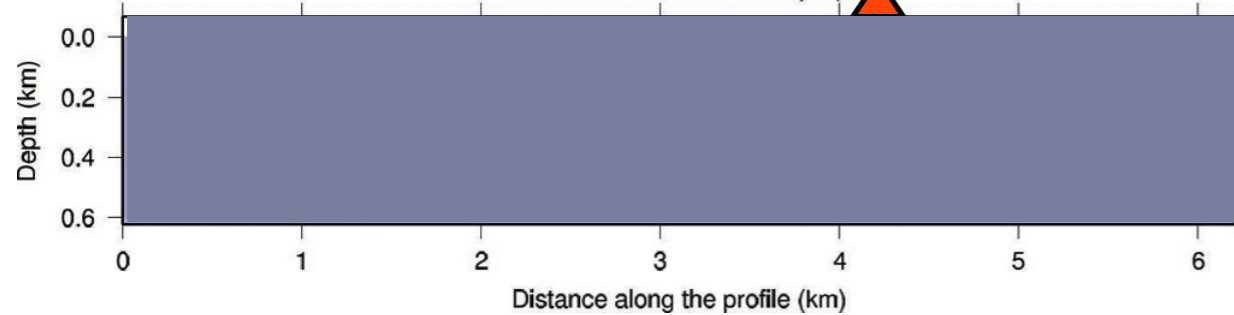
RSR
2D/1D

1D seismogram

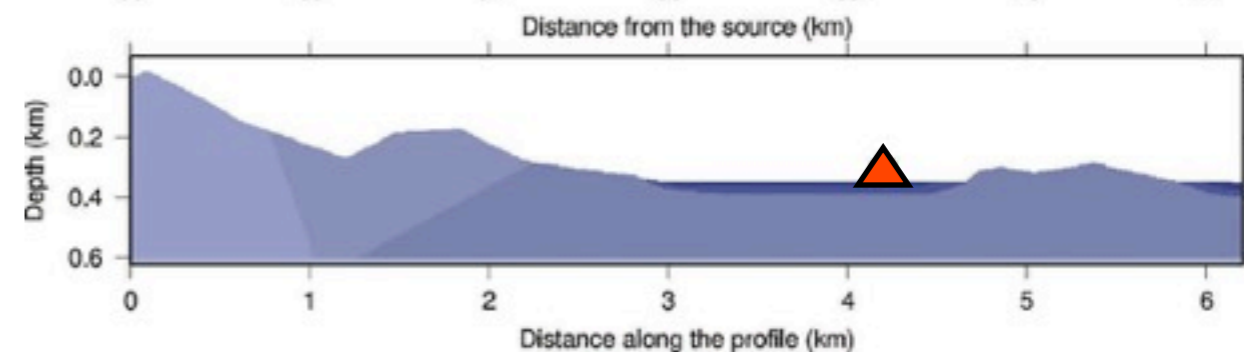
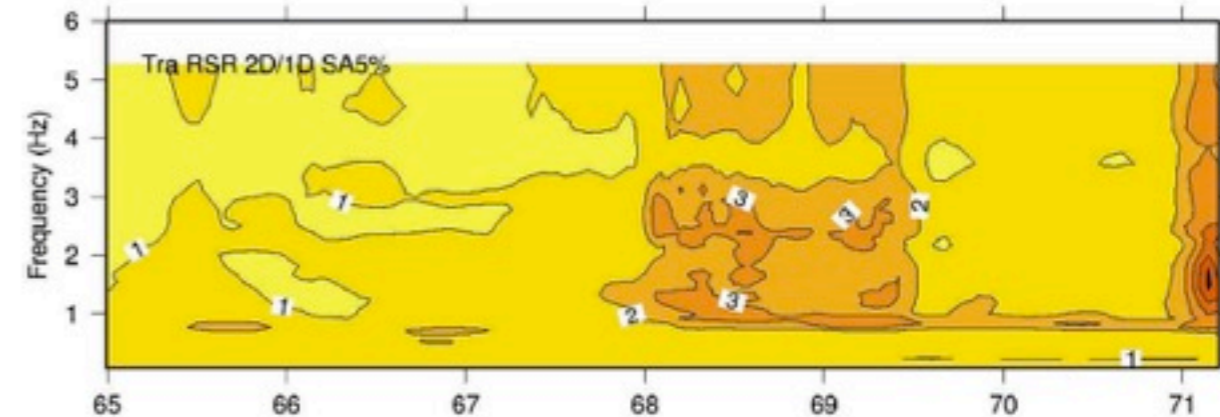
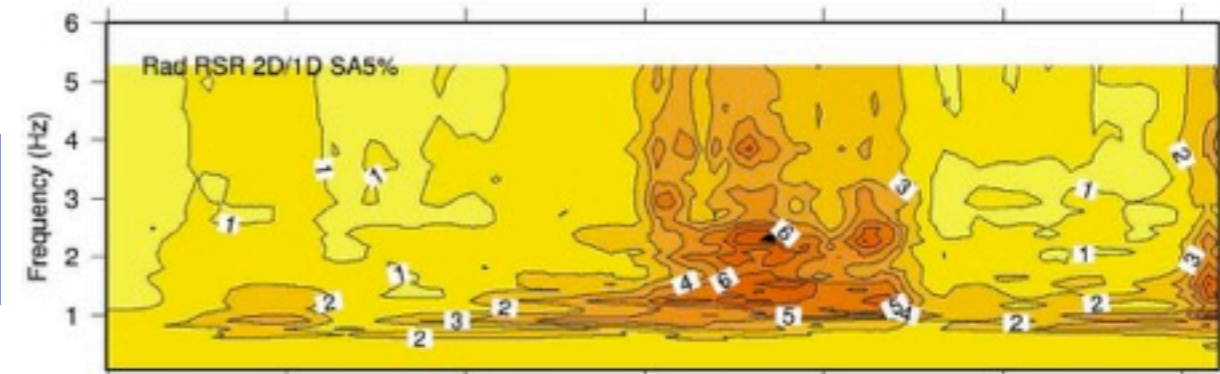
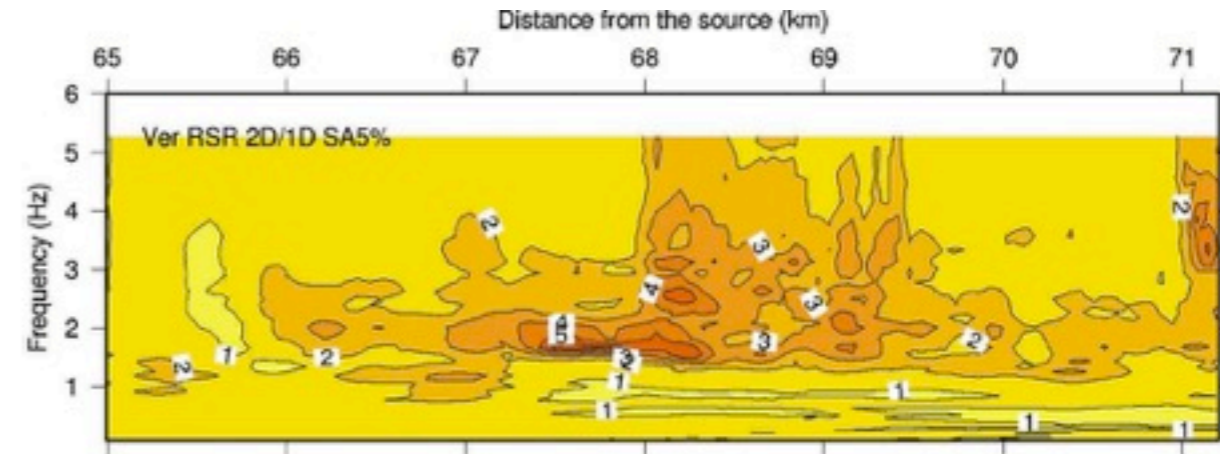
1D response spectra



Bedrock (1D)

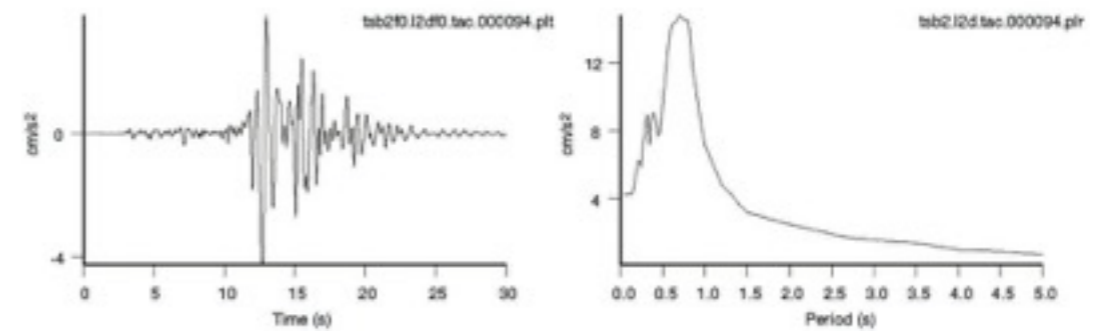
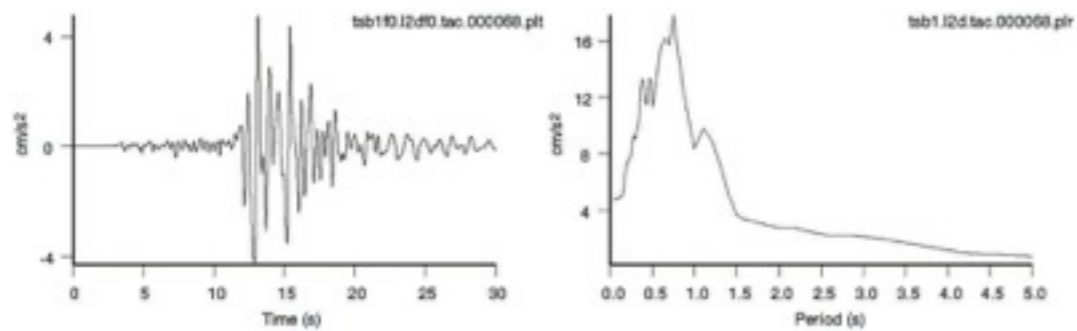
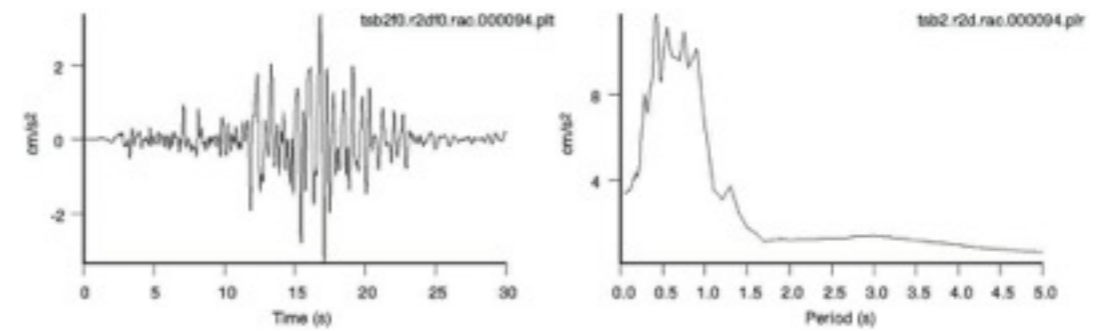
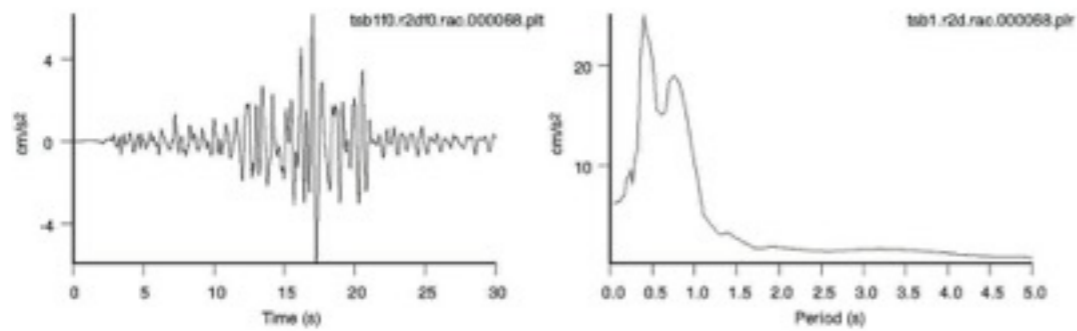
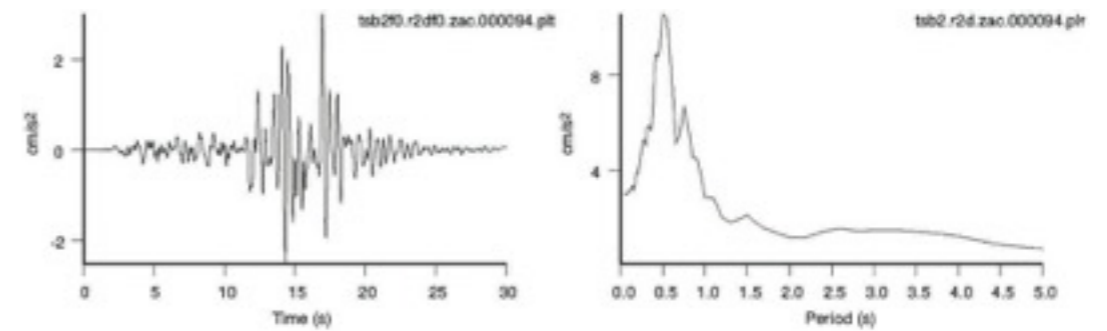
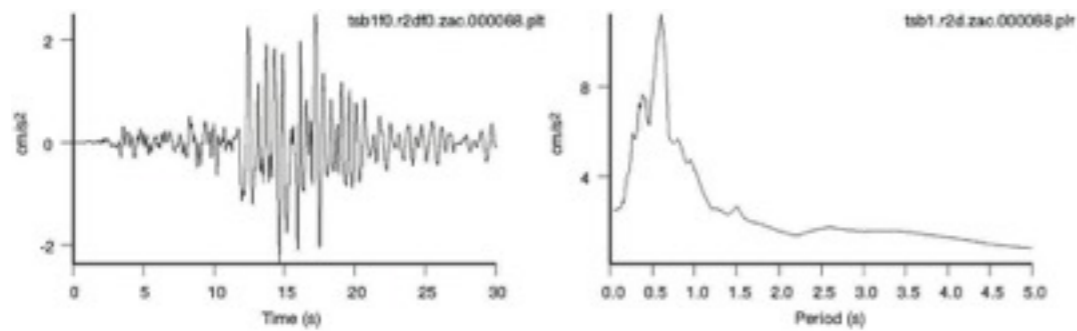


Site effects in Trieste city centre may cause a significant amplification (up to 5 times at engineering relevant frequencies) of the seismic signal at bedrock, hence intensity may reach IX (MCS) or VIII (MSK).



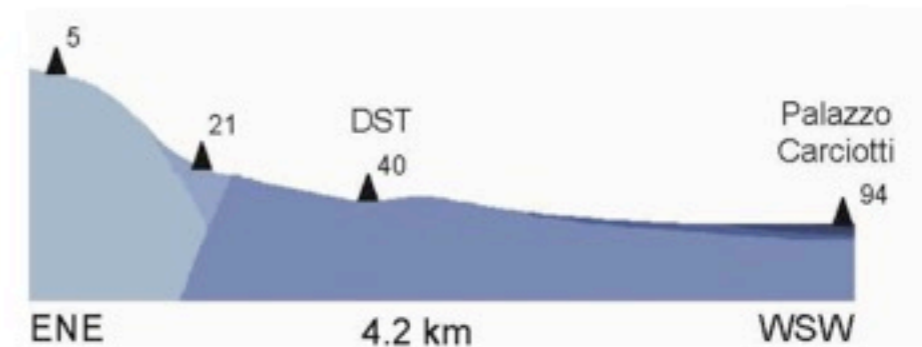
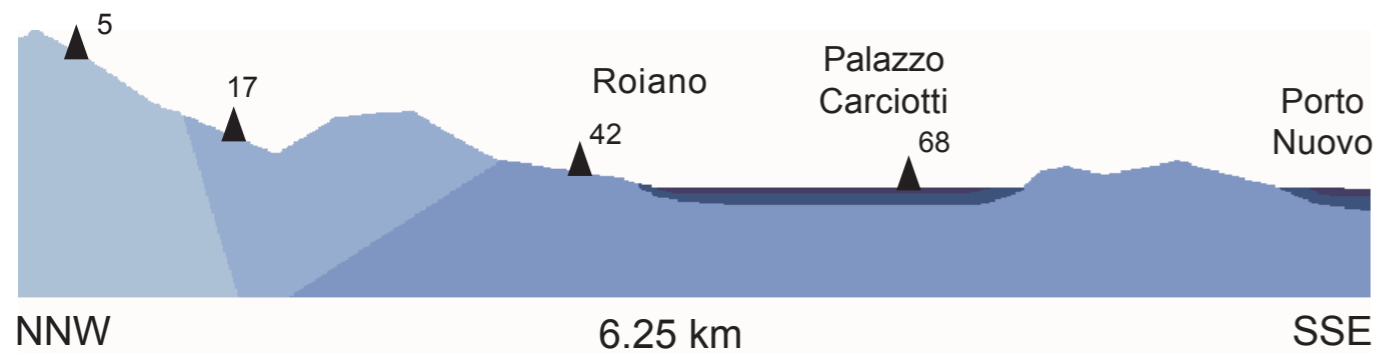
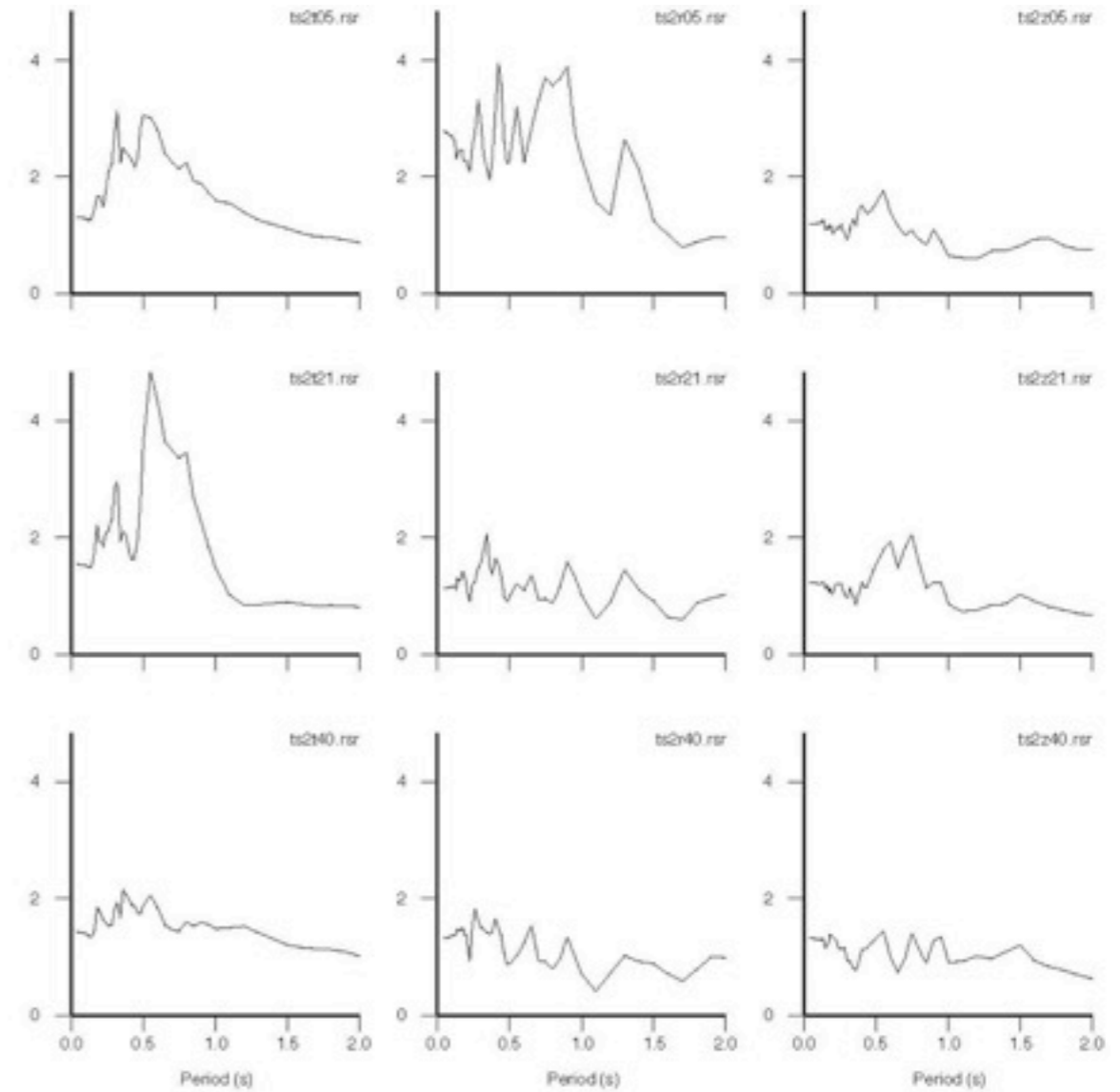
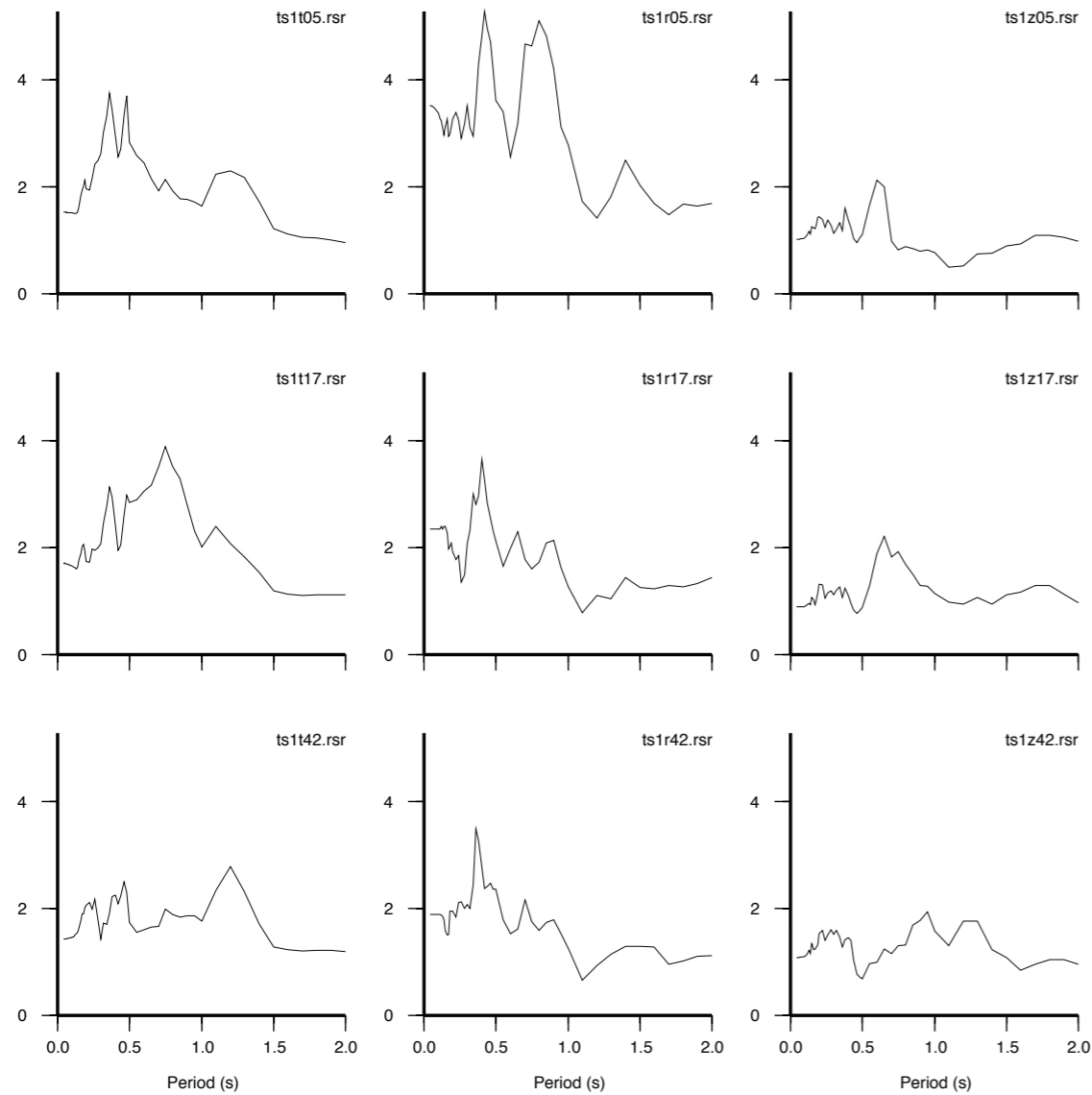
Local Scale - Response Spectra

- Same site at the intersection of two profiles



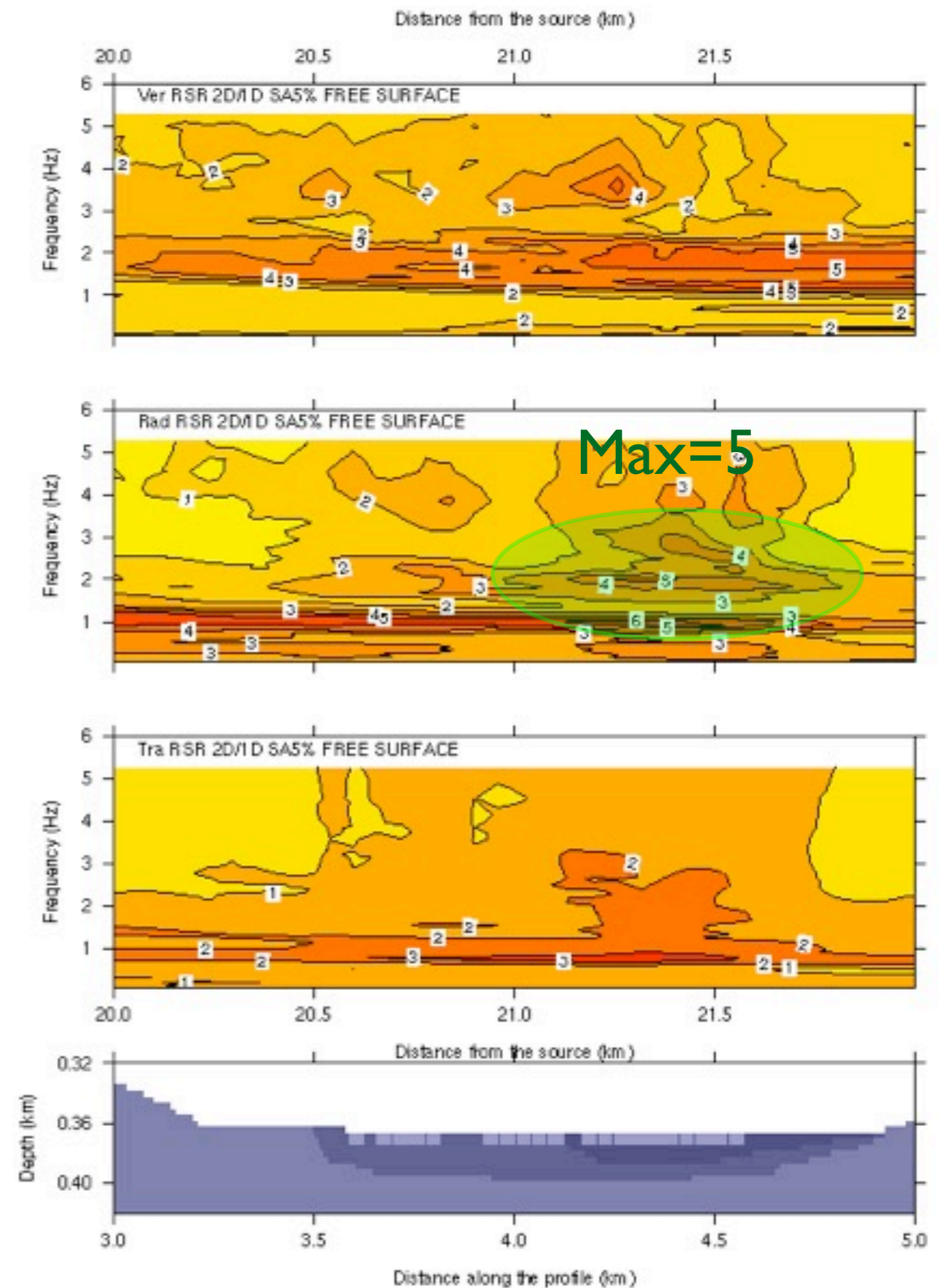
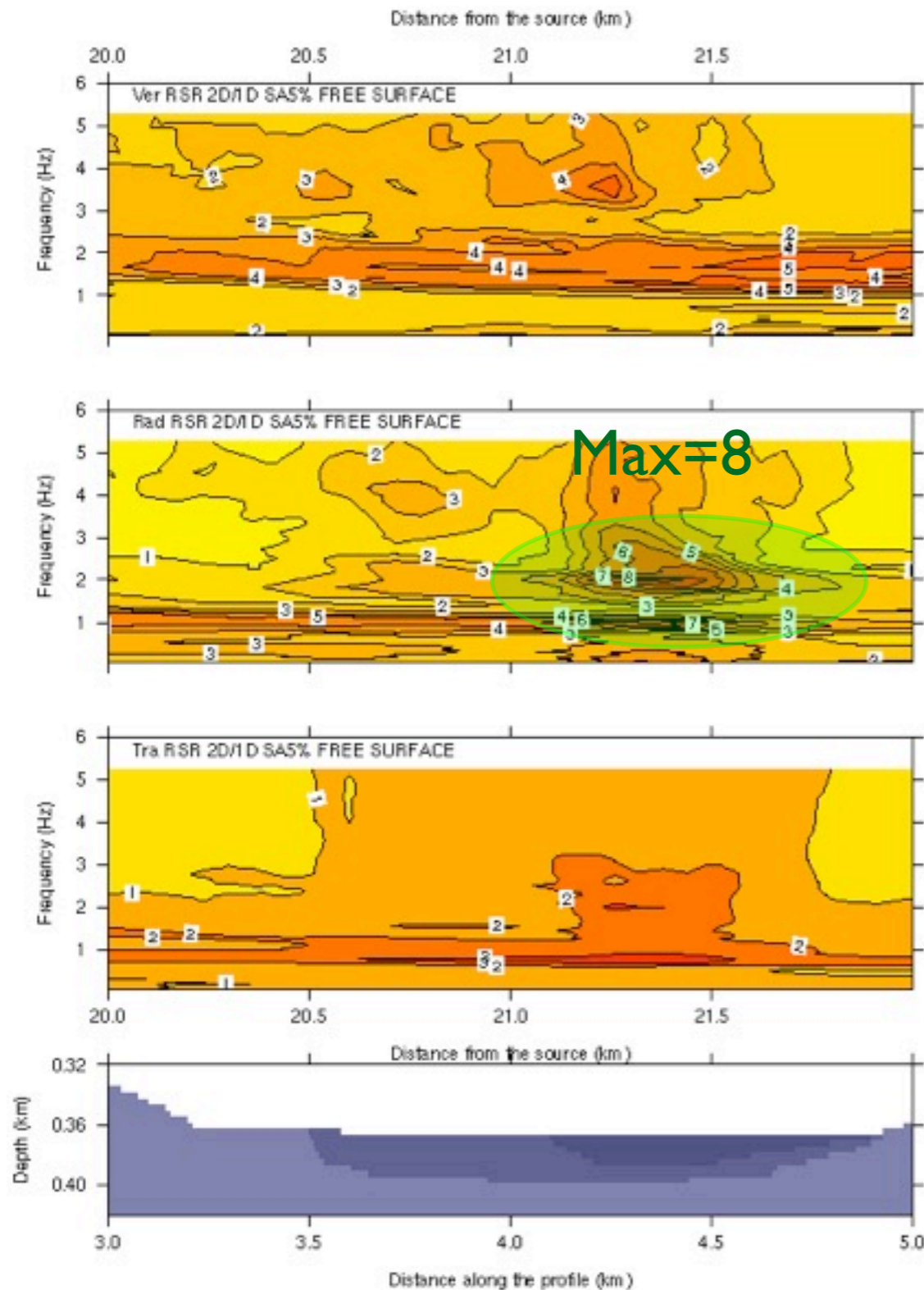
Local Scale - Response Spectra Ratio

Choice of reference site



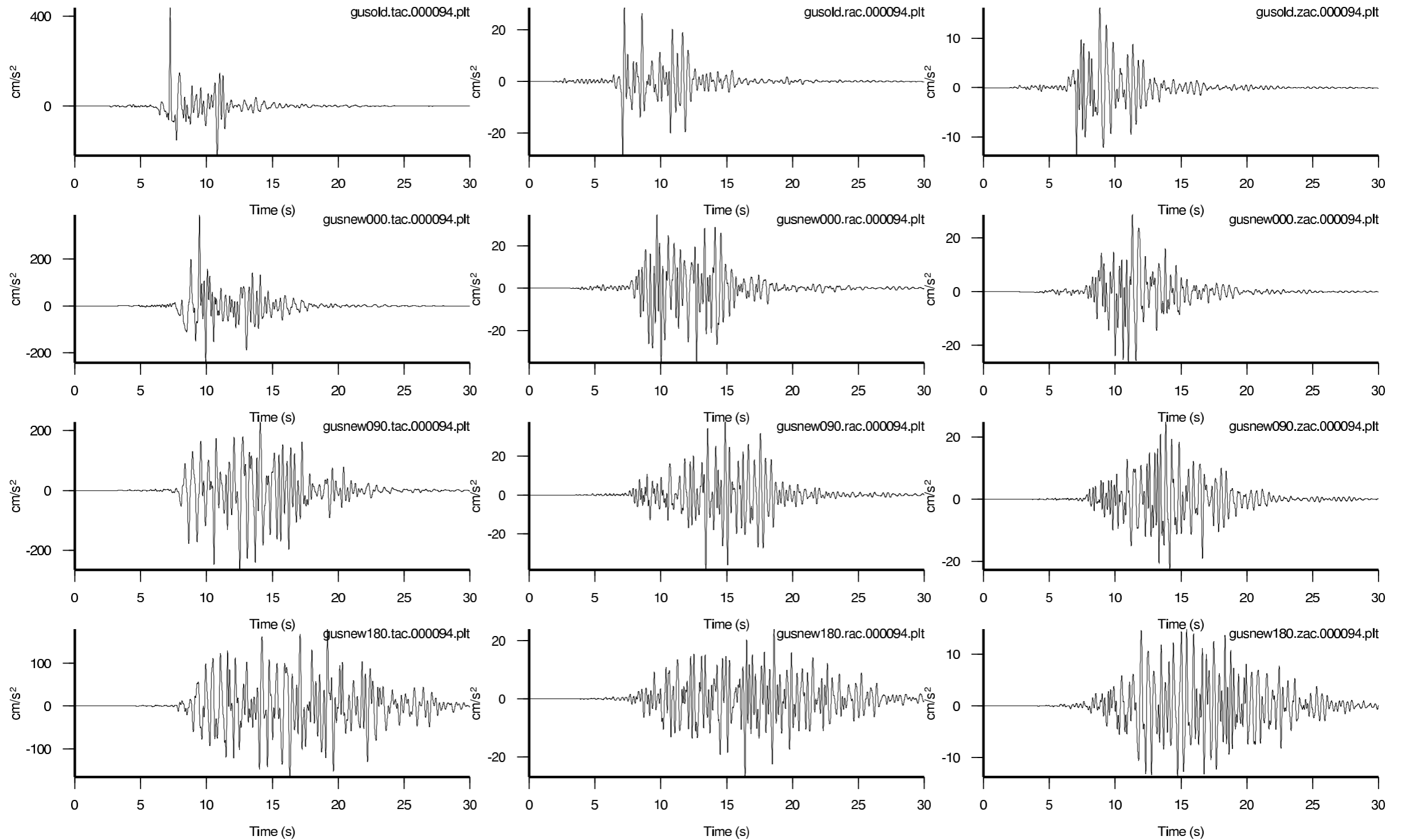
Local Scale - RSR with soil structure interaction

Rive - Dist. 17 km - M=6.0
Foundations and Amplifications (RSR 2D/1D)



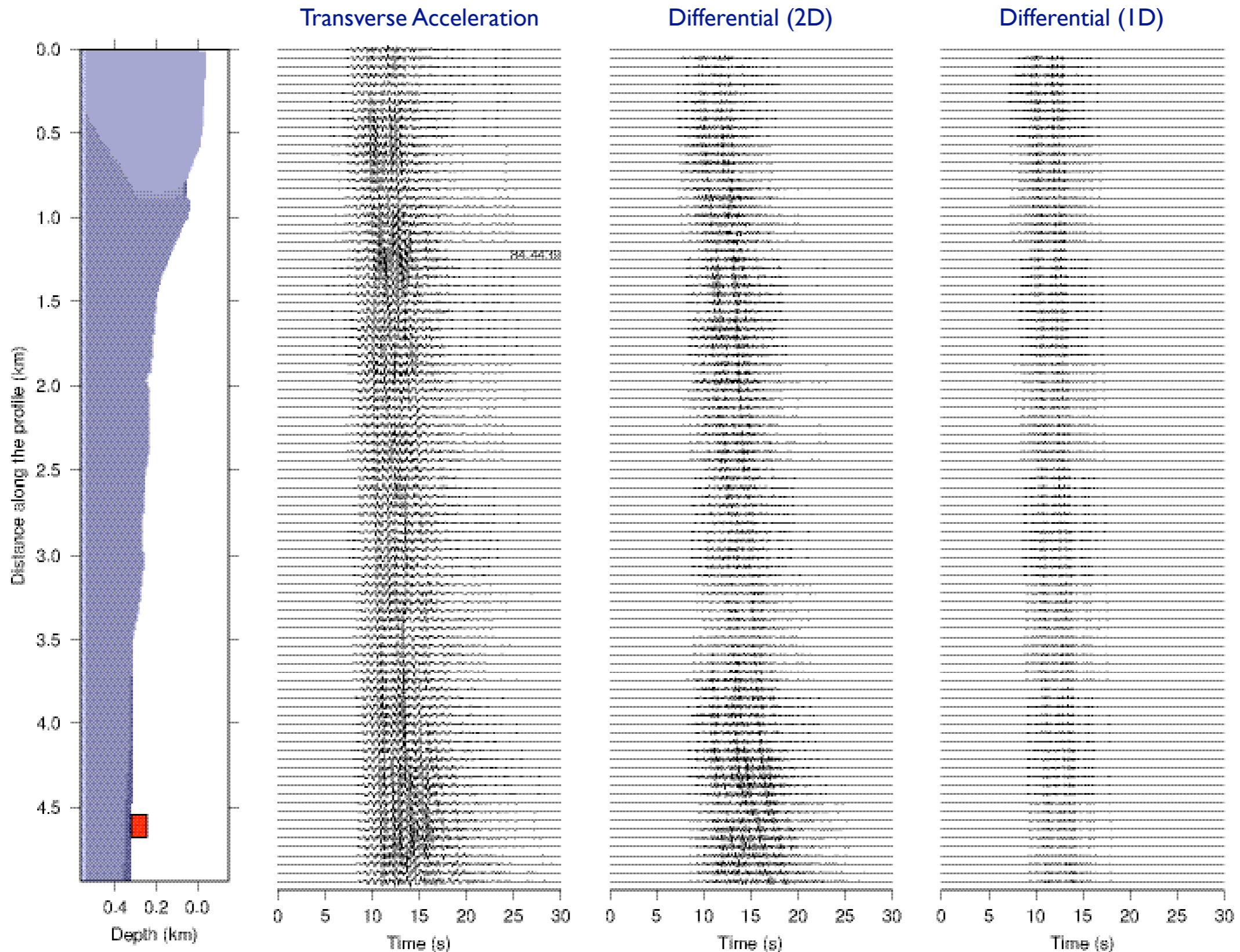
Local Scale - Source Model

● Seismic Source of finite dimension and complicated rupturing process



Local Scale - Differential Motion

- Significant for elongated structures (bridges, lifelines etc)



Engineering analysis - Triest case

- The data set of synthetic seismograms can be fruitfully used and analysed by civil engineers for design and reinforcement actions, and therefore supply a particularly powerful and economical tool for the prevention aspects of Civil Defence.
- Non-linear dynamic analysis considering the seismic input provided by the complete synthetic accelerograms as obtained from microzoning ⇒
Evaluate the response of relevant man-made structures, in terms of displacements and stresses, with respect to a set of possible scenario earthquakes

