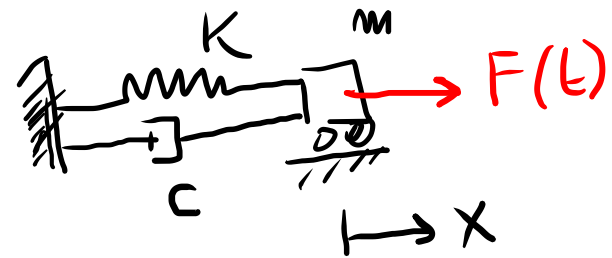
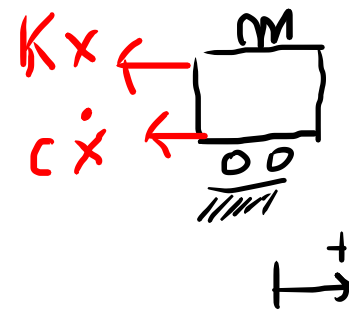


OSCILLATORE CON SMORZAMENTO



OSCILLAZIONI LIBERE
 $F(t) = 0$



c : coefficient di smorzamento viscoso

$c\dot{x}$: FORZA

$$[c] = \left[F \frac{T}{L} \right] \sim \frac{Ns}{m}$$

SISTEMA DISSIPATIVO

EQ. DELLA DINAMICA $m\ddot{x} = -Kx - c\dot{x}$

$$m\ddot{x} + c\dot{x} + Kx = 0 \quad \leadsto \quad \ddot{x} + \underbrace{\frac{c}{m}}_{2v\omega} \dot{x} + \omega^2 x = 0 \quad \leadsto \quad \ddot{x} + 2v\omega \dot{x} + \omega^2 x = 0 \quad (*)$$

$v = \frac{c}{c_c}$: Rapporto di smorzamento ; $c_c = 2m\omega = 2\sqrt{Km}$: smorzamento critico

$v \sim 0.03 \div 0.07$ nelle strutture civili.

$$x(t) = e^{\lambda t}, \quad \dot{x} = \lambda e^{\lambda t}, \quad \ddot{x} = \lambda^2 e^{\lambda t} \Rightarrow (*) \quad (\lambda^2 + 2v\omega\lambda + \omega^2) e^{\lambda t} = 0$$

$$\lambda_{1,2} = -v\omega \pm \sqrt{v^2\omega^2 - \omega^2} = \omega \left(-v \pm \sqrt{v^2 - 1} \right)$$

$v > 1$
 $v = 1$
 $v < 1$

SMORZ. SUPERCRITICO

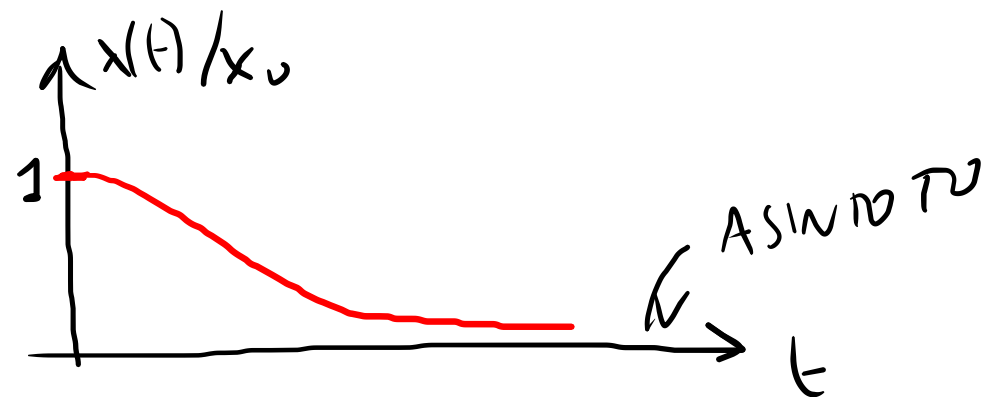
SMORZ. CRITICO

" SOTTOCRITICO

$$v > 1 : x(t) = \underbrace{A_1 e^{\lambda_1 t} + A_2 e^{\lambda_2 t}}_{(*)} = \underbrace{e^{-v\omega t}}_{\text{MODULANTE}} \left[\underbrace{A_1 e^{+\omega\sqrt{v^2-1}t} + A_2 e^{-\omega\sqrt{v^2-1}t}}_{\text{CONDIZ. INIZIALI}} \right]$$



$v = 1 : \lambda_1 = \lambda_2$, la soluz. $(*)$ NON VA PIÙ BENE $\leadsto x(t) = A_1 e^{-v\omega t} + A_2 t e^{-v\omega t}$



$$v < 1: x(t) = A_1 \underbrace{e^{-v\omega t}} + i\omega \underbrace{\sqrt{1-v^2}}_{\omega_D} t + A_2 \underbrace{e^{-v\omega t - i\omega \sqrt{1-v^2} t}}$$

$$\omega_D = \omega \sqrt{1-v^2} \quad \bar{e} \simeq \omega_D$$

per $v \ll 1$

$$= \underbrace{e^{-v\omega t}}_{\text{MODULANTE}} \left(B_1 \sin \omega_D t + B_2 \cos \omega_D t \right)$$

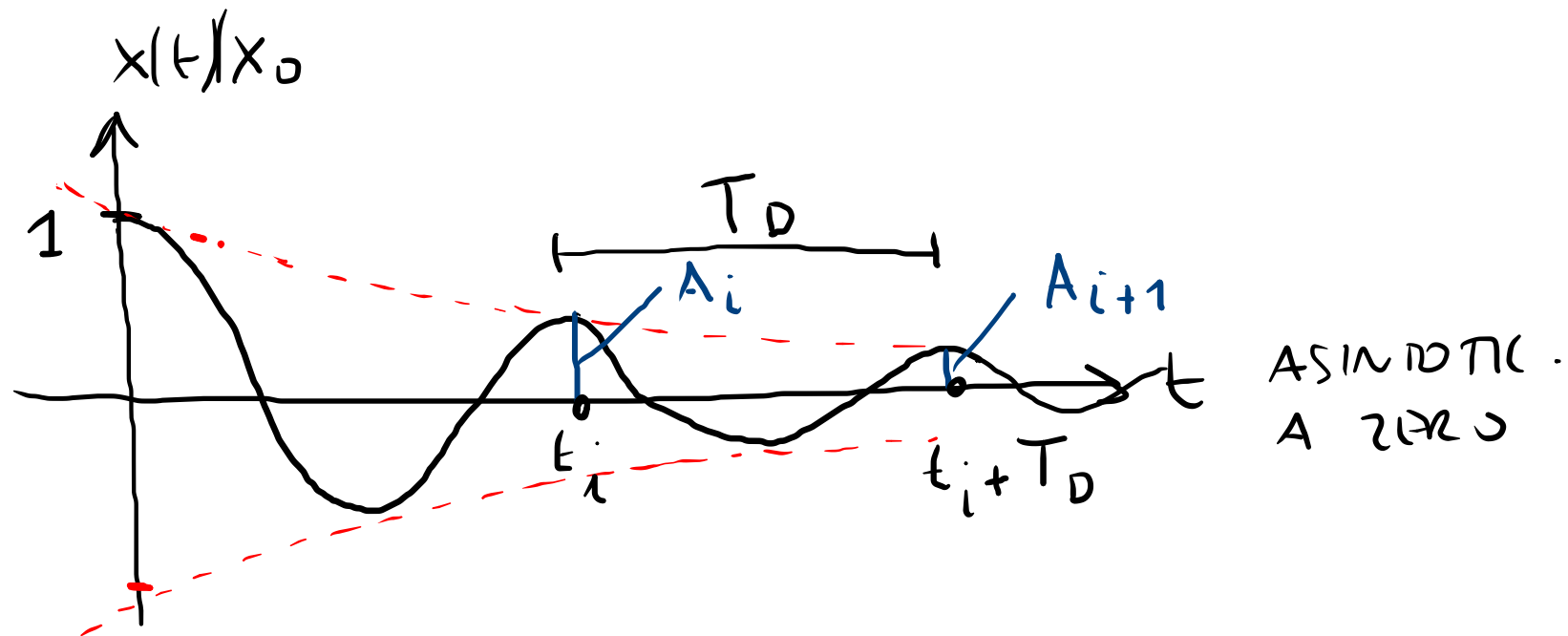
OSCILLANTI

$$T_D = \frac{2\pi}{\omega_D} : \text{periodo "ridotto"}$$

$$\omega_D < \omega ; T_D > T$$

$$v \rightarrow 0, \quad \omega_D = \omega, \quad T_D = T$$

$$v \rightarrow 1, \quad \omega_D \rightarrow 0, \quad T_D \rightarrow \infty$$



Ad ogni ciclo, la perdita di E_{mecc} TOTALE $\bar{e}$ PARL A: $\Delta E_{mecc} = \frac{1}{2} K (A_{i+1}^2 - A_i^2) < 0$

$$A_i = e^{-v\omega t_i}$$

$$A_{i+1} = e^{-v\omega(t_i + T_D)} = e^{-v\omega t_i} \cdot e^{-v\omega T_D} = A_i e^{-v\omega T_D}$$

$$A_i / A_{i+1} = e^{v\omega T_D}$$

$$\frac{A_i}{A_{i+1}} = e^{\underbrace{\nu \omega T_D}_{\delta}} = e^{\delta}$$

$$\delta = \nu \omega \frac{2\pi}{\omega \sqrt{1-\nu^2}} = \frac{2\pi \nu}{1-\nu^2}$$

nel caso $\nu \ll 1$ $\boxed{\delta \approx 2\pi \nu}$

δ : DECREMENTO LOGARITMICO

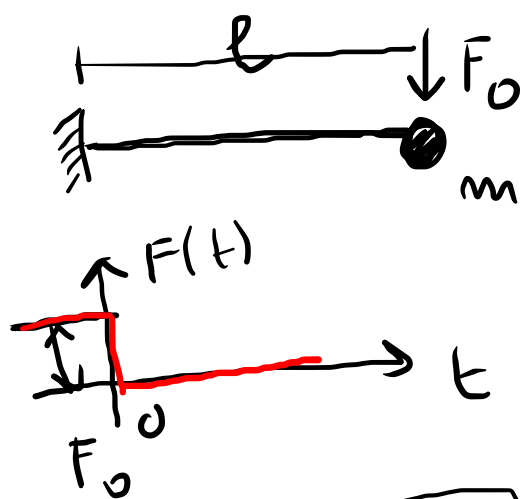
$$\delta = \ln \frac{A_i}{A_{i+1}}$$

$$\boxed{\frac{A_i}{A_{i+1}} \approx 1 + 2\pi \nu}$$

① si ottiene facilmente ν attraverso il rapporto tra 2 ampiezze successive.

$$e^{\delta} \approx e^{2\pi \nu} \approx 1 + 2\pi \nu \quad (\nu \ll 1)$$

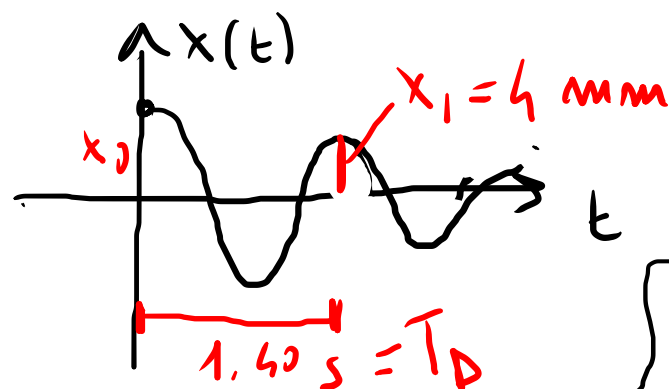
IDENTIFICAZIONE STRUTTURALE STUDIO LE OSCILLAZIONI LIBERE $\longrightarrow$ IL COEFFICIENTE ν



F_0 : FORZA STATICA = 90 KN

$I x_0 = 5 \text{ mm}$

$$\boxed{K x_0 = F_0 + mg}$$



$K?$ $C?$
 $m?$

DA ① ottengo ν

$$\omega = \frac{\omega_p}{\sqrt{1-\nu^2}} = \frac{2\pi}{T_D} \frac{1}{\sqrt{1-\nu^2}} \quad \checkmark$$

$$\omega^2 = \frac{K}{m}$$

$$K x_0 = F_0 + mg$$

$\Rightarrow$ 2 EQ. NELLE INCOGNITE m, K

$$K = \frac{3EI}{l^3}$$