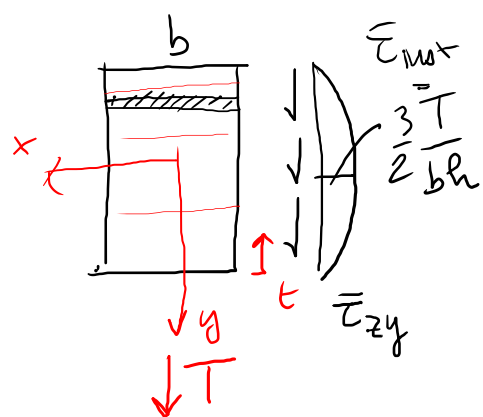
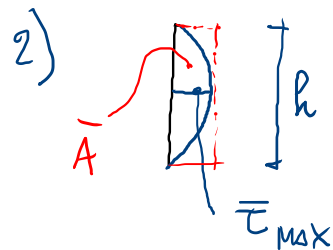




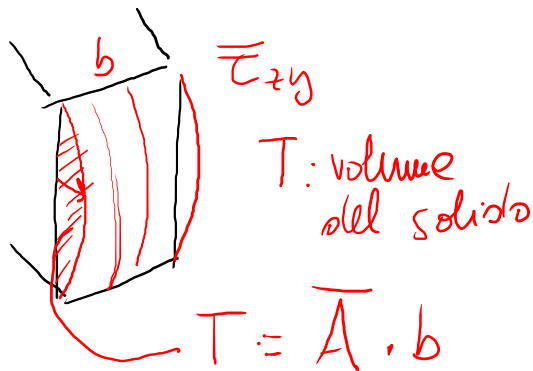
$$\int \bar{\tau}_{zy} dS = T$$

$$1) \int_0^h \bar{\tau}_{zy}(t) b dt = \dots T$$

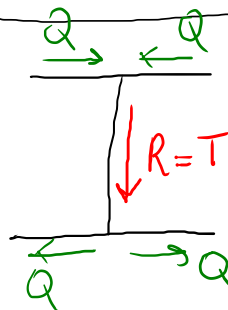


$$\bar{A} = \frac{2}{3} \bar{\tau}_{max} h$$

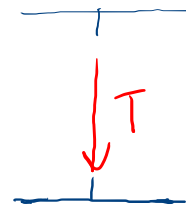
$$T = \frac{2}{3} \bar{\tau}_{max} h b = \frac{2}{3} \frac{3}{2} \frac{T}{b h} h b = T$$



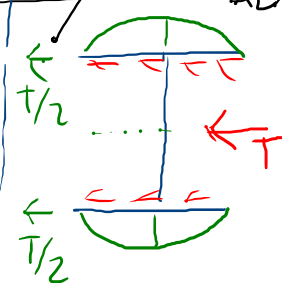
OSS.:



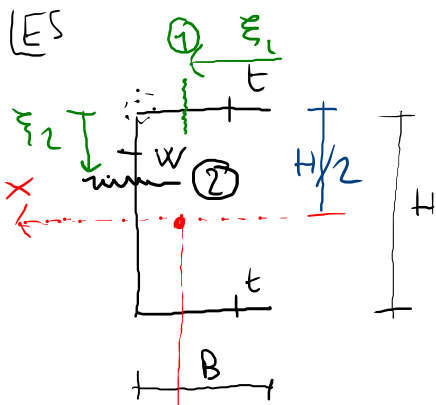
$\Rightarrow$



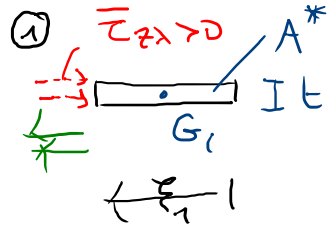
RISULT. SINGOLA ALA



ESERCIZIO



$$T_y \Rightarrow J_x, S_x^* \quad \bar{c} = \frac{T_y}{J_x} \frac{S_x^*}{b}$$



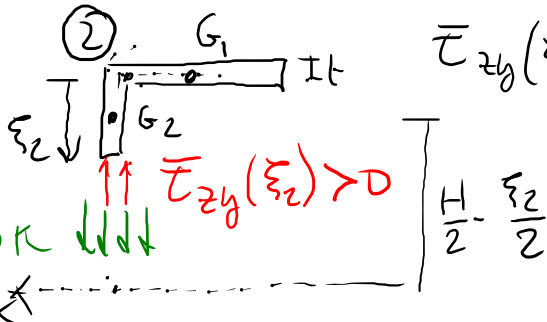
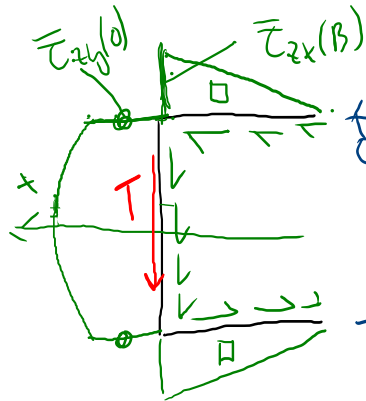
$$\bar{c}_{zx}(\xi_1) = \frac{T}{J_x t} \left(-\frac{H}{2} \cdot \xi_1 t \right) = f(\xi_1) \quad \text{linear}$$

Dopo Facciamo
DELLE CONSID.

$$\bar{c}_{zx}(B) t \quad \bar{c}_{zy}(0) w //$$

UGUAGLI.
DEI
FLUSSI

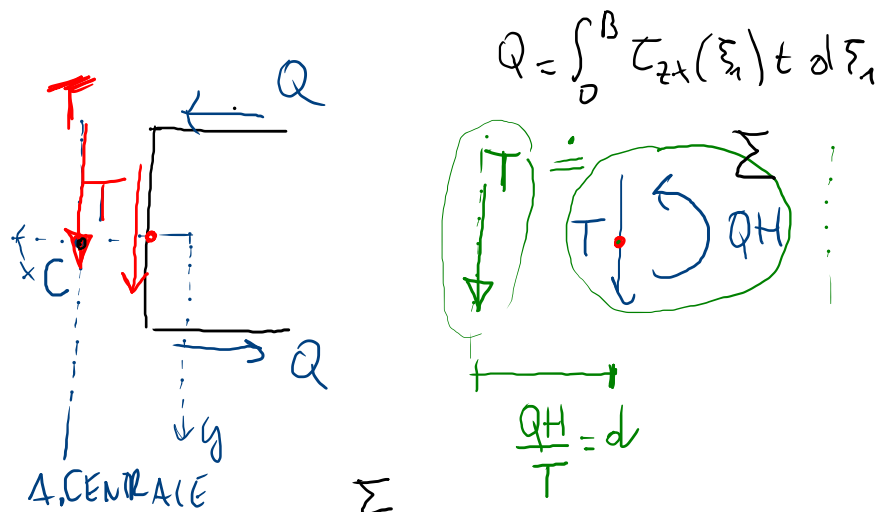
VERIFICARE



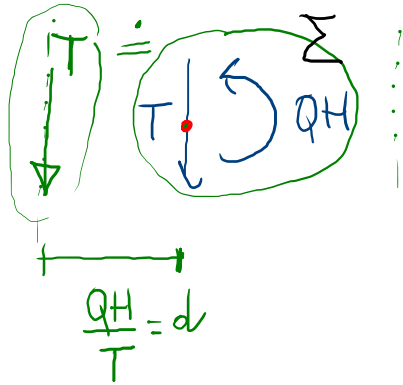
$$\bar{c}_{zy}(\xi_2) = \frac{T}{J_x w} \left[B t \left(-\frac{H}{2} \right) + \xi_2 w \left(-\left(\frac{H}{2} - \frac{\xi_2}{2} \right) \right) \right]$$

< 0

$$\int_0^H \bar{c}_{zy}(\xi_2) w d\xi_2 = T$$



$$Q = \int_0^B \tau_{zx}(\xi_1) t d\xi_1$$



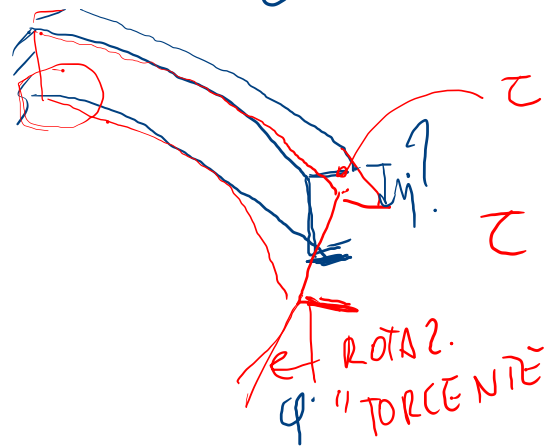
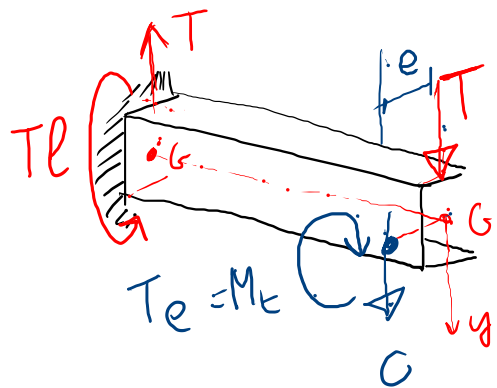
QUALI SONO LE EQUAZ. DI EQUIVALENZA
STANCA A CUI LE $\bar{\tau}_{zx}$, $\bar{\tau}_{zy}$ DEVONO
SOTTOSTARE?

$$\Rightarrow \int \bar{\tau}_{zx} dS = T_x, \quad \int \bar{\tau}_{zy} dS = T_y$$

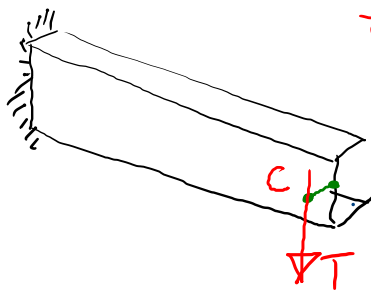
$$?? \quad \int \bar{\tau}_{zy} x - \bar{\tau}_{zx} y dS = M_z$$

IL SIST. $\{T, Q, Q\}$ È EQUIVALENTE A UN TAGLIO T E UN MOMENTO TORCENTE QH .

DOVE DEVO APPLICARE IL TAGLIO T PERCHÉ LA SEZ. NON SUBISCA UNA
TORSIONE (NO MOMENTO TORCENTE!): SULL'ASSE CENTRALE CHE
UNA RETTA ESTERNA PASSANTE PER C (C : CENTRO DI TAGLIO)

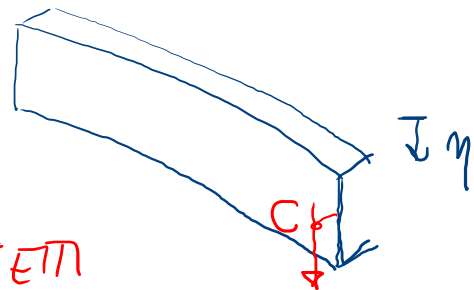


τ JOUROWSKY
+
 τ MOMENT
TORCENIE

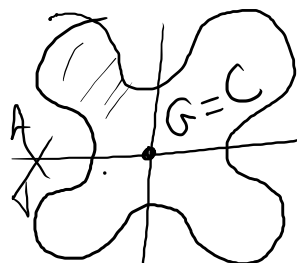


τ : JOUROWSKY

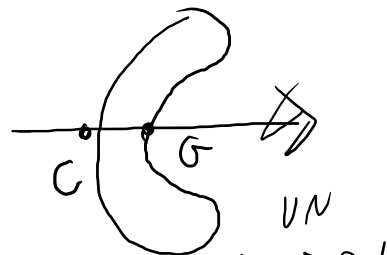
NO EFFETTI
TORCENTI



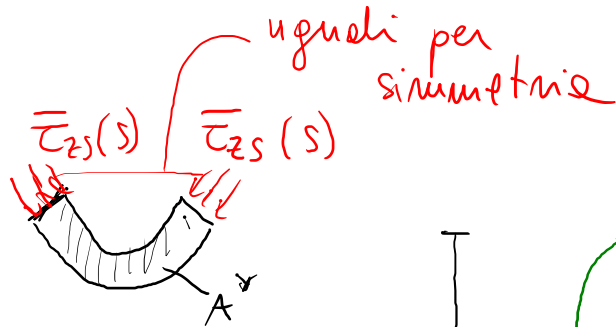
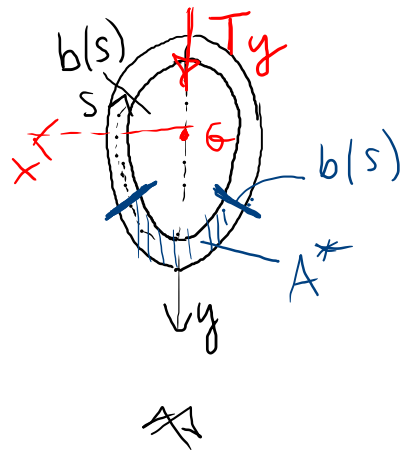
NO ROTAZ



DUE
ASSI DI
SIMMETRIA



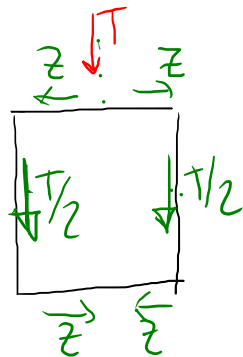
SEZ. SOTTILI CHIUSE CON UN ASSE DI SIMMETRIA



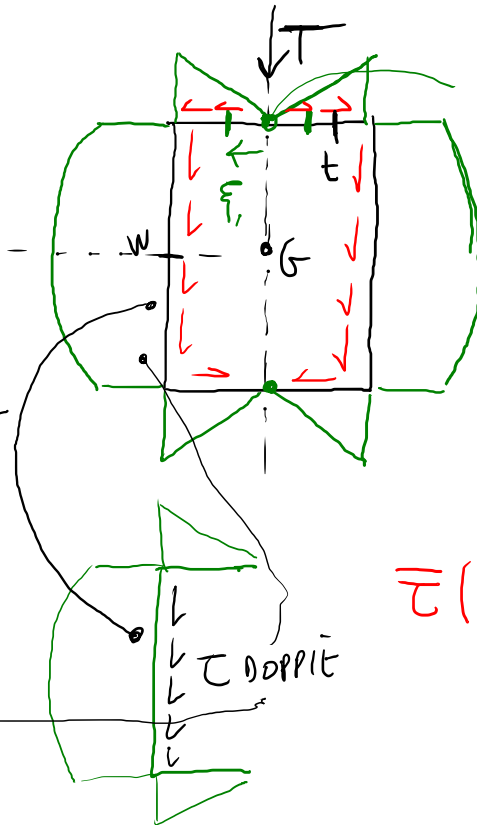
$$\bar{\tau}_{zs}(s) = \frac{T_y}{J_x} \frac{S_x^*}{2b(s)}$$

$$\bar{\tau}(\xi_1=0) = 0$$

$$S_x^* = 0 \quad (A^* = 0)$$

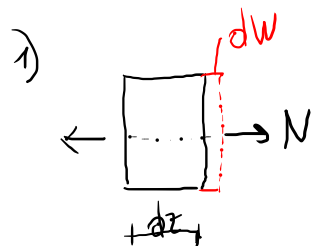


I DUE ANDAMENTI
SONO UGUALI
PERO'
 $J_x = 2 J_x$

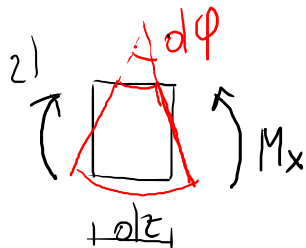


$$\bar{\tau}(\xi_1) = \frac{T_y}{J_x} \frac{2 \xi_1 t (-\frac{H}{2})}{2t}$$

DEFORMAZIONE INDOTTA DAL TAGLIO DEL CONCIO ELEMENTARE (DEL CILINDRO DSV)

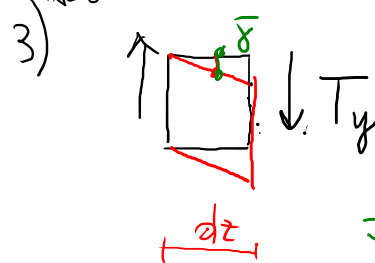


$$\epsilon = \frac{N}{EA}$$

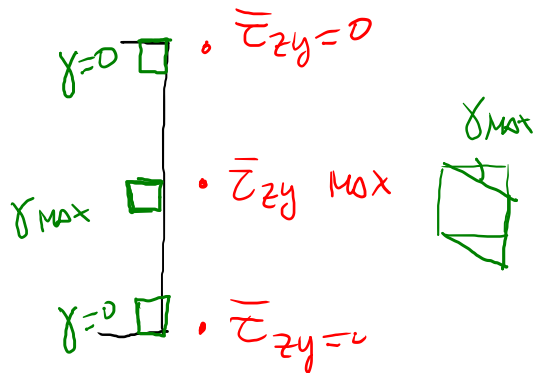
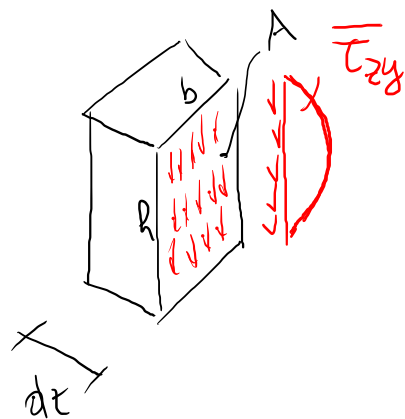


$$\chi = \frac{M_x}{EJ_x}$$

TAGLIO-FLESS.



$$\bar{\gamma} \leftrightarrow T_y$$



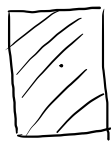
$$\bar{\gamma} = \frac{K T_y}{GA}$$

K : FATTORE DI TAGLIO (ADIMENSIONI).

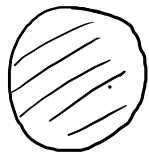
$$K > 1$$

$$\gamma = \frac{\bar{\tau}_{zy}}{G} : \text{LEGGE ELASTICA LINEARE}$$

INGOBBAMENTO DELLA SEZIONE PER SCORRIMENTO DISOMOGENEO



$$K = \frac{6}{5}$$

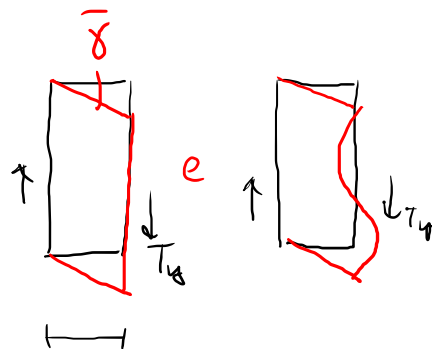


$$K = \frac{32}{27}$$

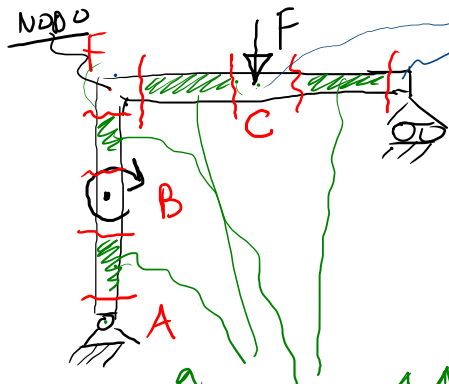
PER CALCOLARE K UTILIZZO UN METODO ENERGETICO: IMPONGO CHE
ABBIAMO LA STESSA ENERGIA ELASTICA (Φ)

$\Rightarrow K$

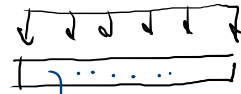
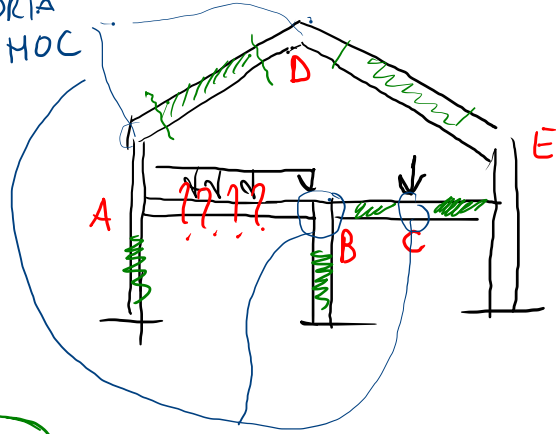
$$\Phi = \int_V (\tau_{zx}^2 + \tau_{zy}^2) \frac{1}{2G} dV \quad \left(\int_V \underline{\sigma} \cdot \underline{\varepsilon} dV \right) + \tau = G\gamma$$



ESTENSIONE DEL PROBL. DSU E TEORIA TECNICA DELLA TRAVE

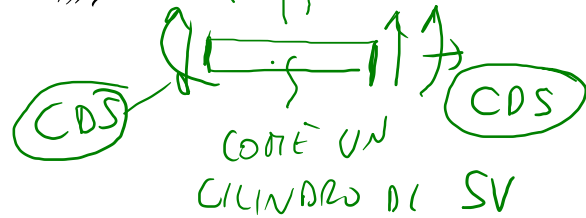


TEORIA
AD HOC



QUI POSSO DIMOSTRARE
CHE LA TEORIA DSU
SI ADATTA BENE
(NAVIER, TOURANSKY)

USO N, T, M DEI VARI
PUNTI



COME UN
CILINDRO DI SV

TEORIA TECNICA DELLA TRAVE: ADOPTA LA TEORIA DEL CILINDRO DSU E LA

APPLICA AI CASI PRATICI UTILI PER LA PROGETTAZIONE. SONO ESCLUSE
LE PARTI DI TRAVE SOLLECITATE PUNTUALMENTE DA CARICHI E MOMENTI
CONCENTRATI E NODI DI STRUTTURE

TEORIA DELLA TRAVE PIANA :



$$\epsilon = \frac{N}{EA}$$



$$\chi = \frac{M}{EJ}$$



$$\bar{\gamma} = \frac{KT}{GA}$$

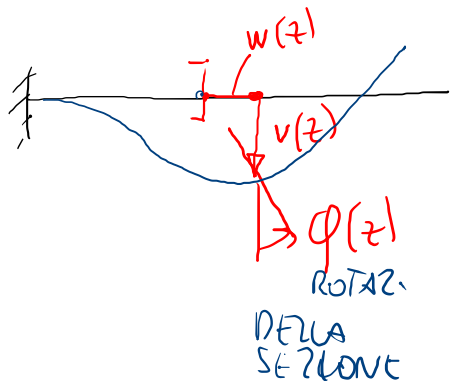
LEGAME COSTITUTIVO
(TRAVE EL. LINEARE)

$$\frac{dN}{dz} = -p(z)$$

$$\frac{dM}{dz} = T(z)$$

$$\frac{dT}{dz} = -q(z)$$

EQUAZ. EQUILIBRIO
PUNTUALE

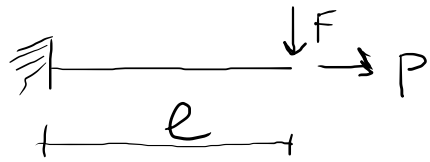


LEGAME TRA
COMPONENTI DI
SPOSTAMENTO DELLA
TRAVE E DEFORMAZIONI

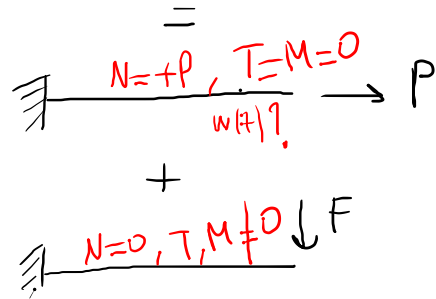
CONGRUENZA

$$w, v, \phi$$

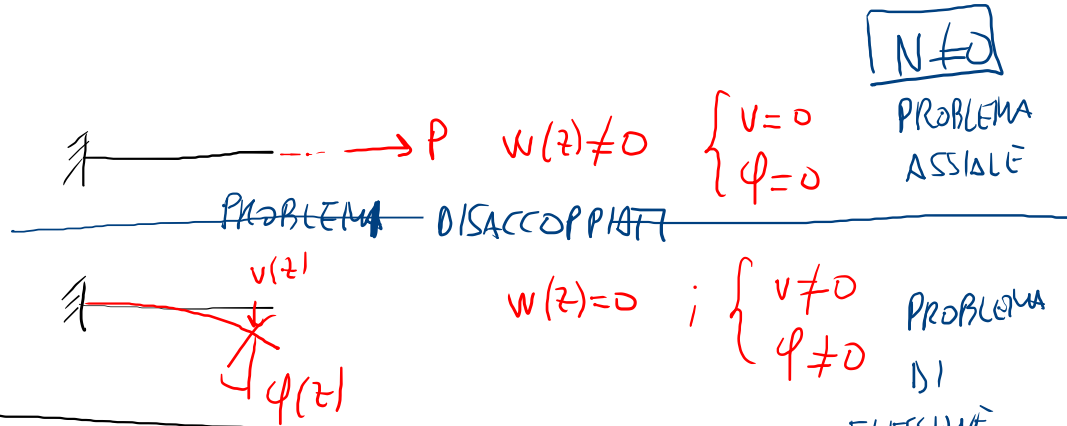
$$\epsilon, \chi, \bar{\gamma}$$



SPOSTAMENTI? $w(z), v(z), \varphi(z) \quad z \in [0, l]$



SOPRAPOSIZIONE
DEGLI
EFFETTI



$N \neq 0$

PROBLEMA
ASSIALE

$$\begin{cases} v=0 \\ \varphi=0 \end{cases}$$

~~PROBLEMA~~ DISACCOPLIATI

$$w(z)=0; \begin{cases} v \neq 0 \\ \varphi \neq 0 \end{cases}$$

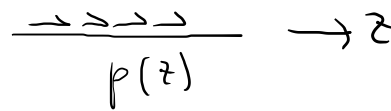
PROBLEMA
DI

FLESSIONE
E SCORRIM.

$M, T \neq 0$

PROBLEMA ASSIALE

EQUILIBRIO: $\frac{dN}{dz} = -p$



CONGRUENZA

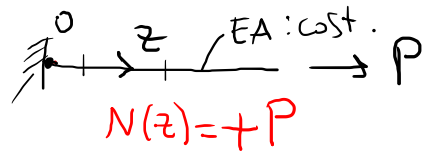
$$\frac{dw}{dz} = \epsilon$$

$$\frac{d(w)}{dz} = \frac{N}{EA} \Rightarrow EA w'(z) = N \Rightarrow N' = (EA w'(z))'$$

$$-p = (EA w''(z))'$$

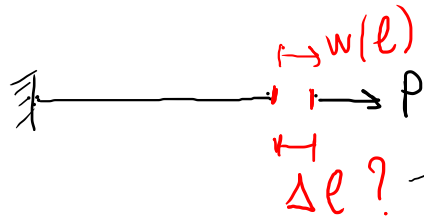
LEG. COST.

$$\epsilon = \frac{N}{EA}$$



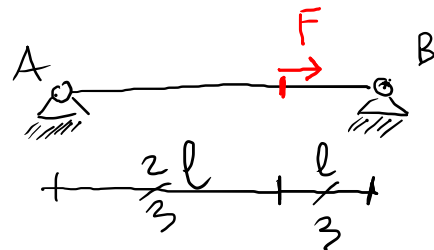
$$\begin{cases} w'(z) = \frac{P}{EA} \\ w(0) = 0 \end{cases}$$

$$\Rightarrow w(z)$$

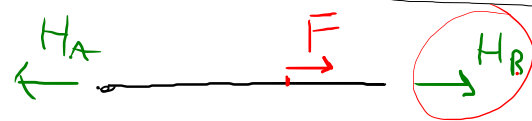


$$\Delta l = \frac{Pl}{EA} : \text{DAL PROBL (1) NSV}$$

UNA STRUTTURA IPERSTATICA



$$\begin{aligned} q &= 3 \\ v &= 4 \\ i &= 1 \\ \infty^1 \text{ SOLUZ. CON } \\ &\text{LE E.C.S.} \end{aligned}$$

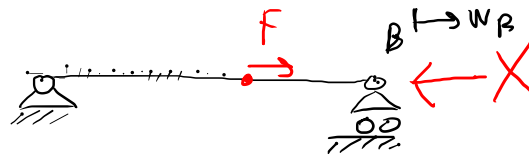


$$\rightarrow : -H_A + F + H_B = 0 \quad 1 \text{ EQUAZ. CON } 2 \text{ INCOGNITE}$$

STAT. UNDET.ERM.

∞^1 SOLUZ.

AGGIUNGO UN' EQUAZ. DI "ELASTICITA"



(INCOGNITA IPERSTATICA)

STR. ISOSTATICA
(STR. PRINCIPALE + S.P.)

$$w_B(F, X) = 0$$

EQ. DI CONGRUENZA