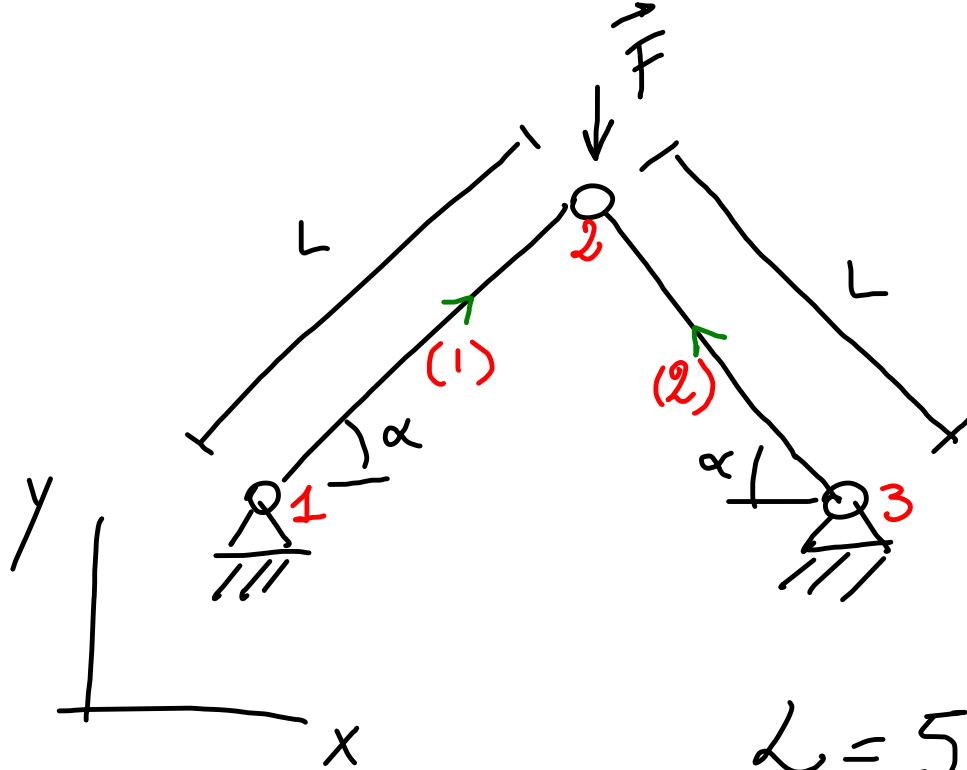


Esercizio : calcolare lo spostamento del nodo 2 della struttura di seguito riportata utilizzando il metodo FEM con assemblaggio diretto della matrice dei coefficienti.



In verde è indicata l'orientazione da assumere per gli elementi.

$$F_x = 0$$

$$F_y = 1000 \text{ kgf}$$

$$L = 5 \text{ m}$$

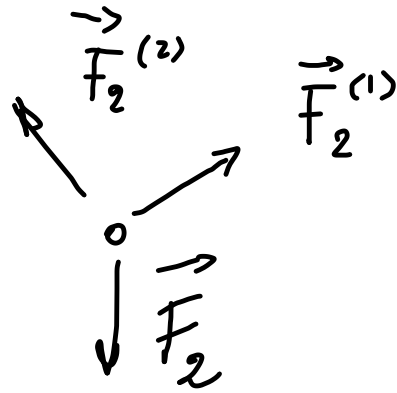
$$E = 11 \text{ GPa}$$

$$\alpha = 45^\circ$$

$$A = 280 \text{ cm}^2$$

Soluzioni

- Equilibrio nodo 2:



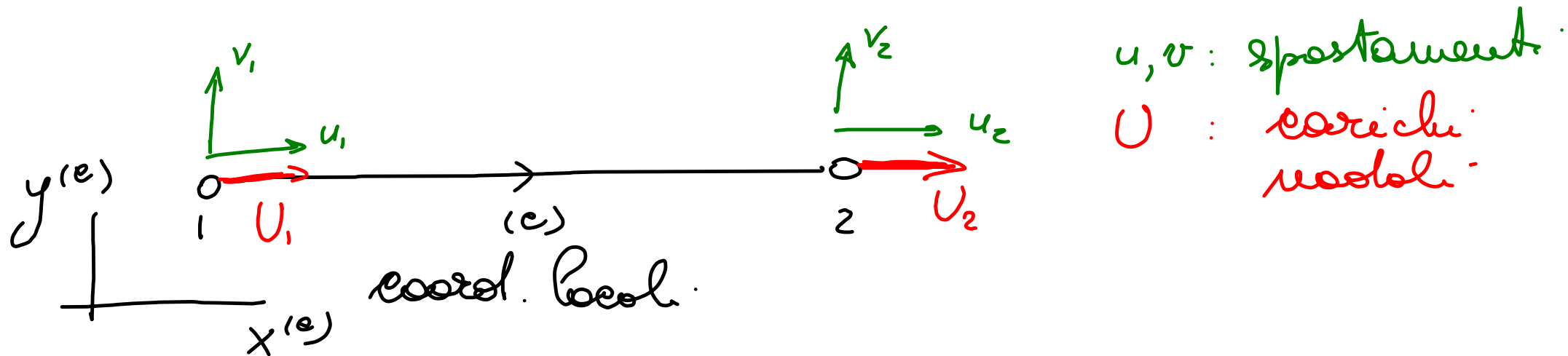
$$\sum_e \vec{F}_2^{(e)} + \vec{F}_2 = \mathbf{0}$$

$\vec{F}_2^{(e)}$: forza esercitata dall'asta (e) sul nodo 2

\vec{F}_2 : forza esterna applicata sul nodo 2.

• Forma esercitata dalle estre ai nodi:

- È conveniente usare un sistema di coordinate locali in quanto i carichi nodali sono puramente assiali e possono essere espressi in modo semplice in funzione degli spostamenti assiali dei nodi.

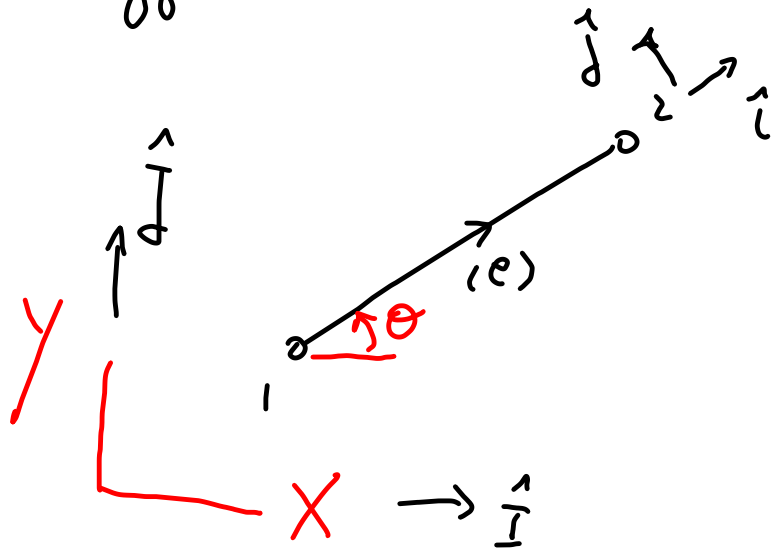


$$U_1 = -EA \varepsilon = -EA \frac{u_2 - u_1}{L}$$

$$U_2 = EA \varepsilon = EA \frac{u_2 - u_1}{L}$$

$$\begin{Bmatrix} U_1 \\ U_2 \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} \Rightarrow \begin{Bmatrix} U_1 \\ 0 \\ U_2 \\ 0 \end{Bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{Bmatrix} = [K] \{u\}$$

- Passaggio da coordinate locali a coordinate globali:



$$\vec{u} = u \hat{i} + v \hat{j} = \tilde{u} \hat{I} + \tilde{v} \hat{J}$$

$$\tilde{u} = u \hat{i} \cdot \hat{I} + v \hat{j} \cdot \hat{I} = u \cos \theta - v \sin \theta$$

$$\tilde{v} = u \hat{i} \cdot \hat{J} + v \hat{j} \cdot \hat{J} = u \sin \theta + v \cos \theta$$

\tilde{u}, \tilde{v} : componenti globali spostamento

$$\begin{Bmatrix} \tilde{u} \\ \tilde{v} \end{Bmatrix} = \begin{bmatrix} \cos\theta & -\sin\theta \\ \sin\theta & \cos\theta \end{bmatrix} \begin{Bmatrix} u \\ v \end{Bmatrix} \quad \text{e} \quad \begin{Bmatrix} u \\ v \end{Bmatrix} = \begin{bmatrix} \cos\theta & \sin\theta \\ -\sin\theta & \cos\theta \end{bmatrix} \begin{Bmatrix} \tilde{u} \\ \tilde{v} \end{Bmatrix}$$

Analogamente per i carichi nodali:

$$\begin{Bmatrix} \tilde{U} \\ \tilde{V} \end{Bmatrix} = \begin{bmatrix} \cos\theta & -\sin\theta \\ \sin\theta & \cos\theta \end{bmatrix} \begin{Bmatrix} U \\ 0 \end{Bmatrix}$$

Considerando anche i nodi di un'asta:

$$\begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{Bmatrix} = \begin{bmatrix} [R] & [0] \\ [0] & [R] \end{bmatrix} \begin{Bmatrix} \tilde{u}_1 \\ \tilde{v}_1 \\ \tilde{u}_2 \\ \tilde{v}_2 \end{Bmatrix} \quad \text{e} \quad \begin{Bmatrix} \tilde{U}_1 \\ \tilde{V}_1 \\ \tilde{U}_2 \\ \tilde{V}_2 \end{Bmatrix} = \begin{bmatrix} [R]^T & [0] \\ [0] & [R]^T \end{bmatrix} \begin{Bmatrix} U_1 \\ 0 \\ U_2 \\ 0 \end{Bmatrix}$$

dove

$$[R] = \begin{bmatrix} \cos\theta & \sin\theta \\ -\sin\theta & \cos\theta \end{bmatrix}$$

Indichiamo con $[L]$ la matrice di rotazione dell'asta:

$$[L] = \begin{bmatrix} [R] & [0] \\ [0] & [R] \end{bmatrix}$$

Si ha:

$$\{u\} = [L] \{\tilde{u}\} \quad \{\tilde{U}\} = [L]^T \{U\} \quad \text{dove } \{U\} = \begin{Bmatrix} U_1 \\ 0 \\ U_2 \\ 0 \end{Bmatrix}$$

- A livello di elemento il legame tra carichi nodali e spostamenti nodali in coordinate globali è quindi:

$$\begin{Bmatrix} U_1 \\ 0 \\ U_2 \\ 0 \end{Bmatrix} = [K] \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{Bmatrix} \Rightarrow \begin{Bmatrix} \tilde{U}_1 \\ \tilde{V}_1 \\ \tilde{U}_2 \\ \tilde{V}_2 \end{Bmatrix} = [L]^T \begin{Bmatrix} U_1 \\ 0 \\ U_2 \\ 0 \end{Bmatrix} = [L]^T [K] [L] \begin{Bmatrix} \tilde{u}_1 \\ \tilde{v}_1 \\ \tilde{u}_2 \\ \tilde{v}_2 \end{Bmatrix} \\ = [\tilde{K}] \begin{Bmatrix} \tilde{u}_1 \\ \tilde{v}_1 \\ \tilde{u}_2 \\ \tilde{v}_2 \end{Bmatrix}$$

• Deduzione $[K]$:

$$[L] = \begin{bmatrix} \begin{bmatrix} c & \lambda \\ -\lambda & c \end{bmatrix} & \begin{bmatrix} 0 & 0 \\ 0 & 0 \end{bmatrix} \\ \begin{bmatrix} 0 & 0 \\ 0 & 0 \end{bmatrix} & \begin{bmatrix} c & \lambda \\ -\lambda & c \end{bmatrix} \end{bmatrix}$$

$$[K] = \frac{EA}{L} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

$$[L]^T [K] = \frac{EA}{L} \begin{bmatrix} c & 0 & -c & 0 \\ \lambda & 0 & -\lambda & 0 \\ -c & 0 & c & 0 \\ -\lambda & 0 & \lambda & 0 \end{bmatrix};$$

$$[L]^T [K] [L] = \frac{EA}{L} \begin{bmatrix} c^2 & \lambda c & -c^2 & -\lambda c \\ c \lambda & \lambda^2 & -\lambda c & -\lambda^2 \\ -c^2 & -\lambda c & c^2 & \lambda c \\ -c \lambda & -\lambda^2 & c \lambda & \lambda^2 \end{bmatrix}$$

- Anambloggio motica globale:

Siccome dobbiamo fare i calcoli "a mano", consideriamo direttamente il sistema ridotto, con incognite \tilde{u}_2 e \tilde{v}_2 ("2" modo globale):

- Equilibrio modo 2 lungo X:

$$-\tilde{U}_2^{(1)} - \tilde{U}_2^{(2)} + F_X = 0$$

forza agente sul
modo locale 2 dell'asta
1

forza agente sul modo
locale 2 dell'asta 2.

$$- \tilde{K}_{31}^{(1)} \cancel{\tilde{u}_1^{(1)}} - \tilde{K}_{32}^{(1)} \cancel{V_1^{(1)}} - \tilde{K}_{33}^{(1)} \tilde{u}_2^{(1)} - \tilde{K}_{34}^{(1)} \tilde{V}_2^{(1)}$$

$$- \tilde{K}_{31}^{(2)} \cancel{\tilde{u}_1^{(2)}} - \tilde{K}_{32}^{(2)} \cancel{V_1^{(2)}} - \tilde{K}_{33}^{(2)} \tilde{u}_2^{(2)} - \tilde{K}_{34}^{(2)} \tilde{V}_2^{(2)} + F_x = 0$$

$$\tilde{u}_2^{(1)} = \tilde{u}_2^{(2)} \equiv \tilde{u}$$

$$\tilde{u}_1^{(1)} = \tilde{V}_1^{(1)} = \tilde{u}_1^{(2)} = \tilde{V}_1^{(2)} = 0$$

$$\tilde{V}_2^{(1)} = \tilde{V}_2^{(2)} \equiv \tilde{V}$$

$$\Rightarrow - \tilde{K}_{33}^{(1)} \tilde{u} - \tilde{K}_{34}^{(1)} \tilde{V} - \tilde{K}_{33}^{(2)} \tilde{u} - \tilde{K}_{34}^{(2)} \tilde{V} + F_x = 0$$

$$\Rightarrow \left(\tilde{K}_{33}^{(1)} + \tilde{K}_{33}^{(2)} \right) \tilde{u} + \left(\tilde{K}_{34}^{(1)} + \tilde{K}_{34}^{(2)} \right) \tilde{V} = F_x$$

• Equilibrio nodo 2 lungo y :

$$-\tilde{V}_2^{(1)} - \tilde{V}_2^{(2)} + F_y = 0$$

forza agente sul
nodo locale 2 dell'asta
1

forza agente sul nodo
locale 2 dell'asta 2.

$$\left(\tilde{K}_{43}^{(1)} + \tilde{K}_{43}^{(2)} \right) \tilde{u} + \left(K_{44}^{(1)} + K_{44}^{(2)} \right) \tilde{v} = \frac{F_y}{\gamma}$$

$$[\tilde{k}] = \frac{EA}{L} \begin{bmatrix} c^2 & sc & -c^2 & -sc \\ cs & s^2 & -sc & -s^2 \\ -c^2 & -sc & c^2 & sc \\ -cs & -s^2 & cs & s^2 \end{bmatrix}$$

Por elemento 1:

$$c \equiv \cos d$$

$$s \equiv \sin d$$

Por elemento 2:

$$c \equiv \cos(\pi - d) = -\cos d$$

$$s \equiv \sin(\pi - d) = \sin d$$

Ej. Puzo x:

$$\frac{EA}{L} \left\{ \left(\tilde{k}_{33}^{(1)} + \tilde{k}_{33}^{(2)} \right) \tilde{u} + \left(\tilde{k}_{34}^{(1)} + \tilde{k}_{34}^{(2)} \right) \tilde{v} \right\} = F_x$$

$$\left(c_d^2 + (-c_d)^2 \right) \tilde{u} + \left(sc_d + s_d(-c_d) \right) \tilde{v} = \frac{F_x L}{EA}$$

$$2(\cos \alpha)^2 \tilde{u} = F_x L / EA$$

$$\Rightarrow \tilde{u} = 0$$

$$[\tilde{k}] = \frac{EA}{L} \begin{bmatrix} c^2 & sc & -c^2 & -sc \\ cs & s^2 & -sc & -s^2 \\ -c^2 & -sc & c^2 & sc \\ -cs & -s^2 & cs & s^2 \end{bmatrix}$$

Por elemento 1:

$$c \equiv \cos d$$

$$s \equiv \sin d$$

Por elemento 2:

$$c \equiv \cos(\pi - d) = -\cos d$$

$$s \equiv \sin(\pi - d) = \sin d$$

Ej. Puzo y:

$$\left(\tilde{k}_{43}^{(1)} + \tilde{k}_{43}^{(2)} \right) \tilde{u} + \left(\tilde{k}_{44}^{(1)} + \tilde{k}_{44}^{(2)} \right) \tilde{v} = F_y L / EA$$

$$\left(\cos d \sin d + (-\cos d) \sin d \right) \tilde{u} + \left((\sin d)^2 + (\sin d)^2 \right) \tilde{v} = F_y L / EA$$

$$2 (\sin d)^2 \tilde{v} = F_y L / EA$$

$$2 \cdot (\sin 45^\circ)^2 \tilde{V} = - \frac{1000 \times 9.81 \text{ N} \times 5 \text{ m}}{11 \text{ GPa} \times 280 \text{ cm}^2}$$

$$2 \cdot \frac{1}{2} \tilde{V} = \tilde{V} = - \frac{9.81 \times 1000 \times 5}{11 \times 10^9 \times 280 \times 10^{-4}} =$$

$$= - \frac{9.81 \times 5}{11 \times 10^2 \times 280} = - \frac{9.81}{11 \cdot 5600} \approx 0,16 \text{ mm}$$

Sforzo sulle travi :

$$U_2^{(1)} = - k_{33}^{(1)} \tilde{u} - k_{35} \tilde{v} = \left(-\frac{1}{2} \tilde{u} - \frac{1}{2} \tilde{v} \right) \frac{EA}{L} = -4905 \text{ N}$$

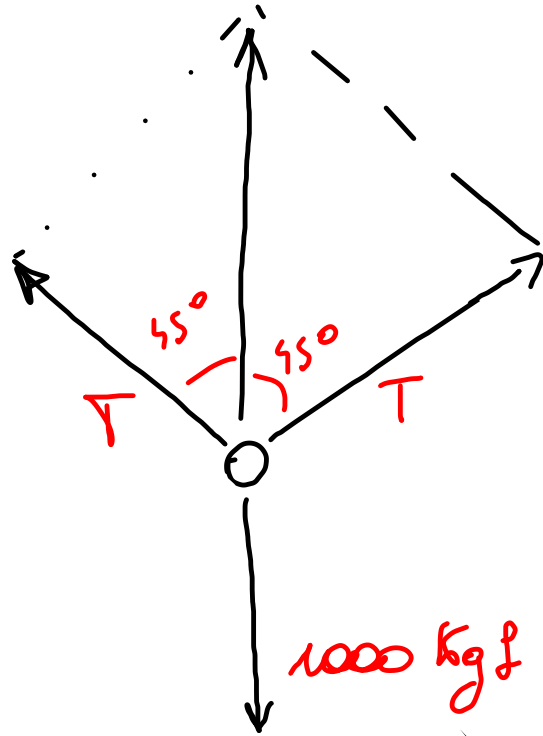
$$V_2^{(1)} = \dots \dots \dots = -500 \text{ kN} \quad \text{kgl}$$

$$V_2^{(1)} = \dots \dots \dots = -500 \text{ kN}$$

$$\{U\} = [R]^T \{\tilde{U}\} \Rightarrow U_2'' = -707,107 \text{ kgf}$$

$$V_2'' = 0 \text{ kgf}$$

• Verifica resultados :



$$2T \frac{\sqrt{2}}{2} = 1000 \text{ kgf}$$

$$T = \frac{1000}{\sqrt{2}} \text{ kgf} = 707,107 \text{ kgf}$$

compressive