

HYDROST. STRESS AND DEVIAIDRIC STRESS

13/3/25

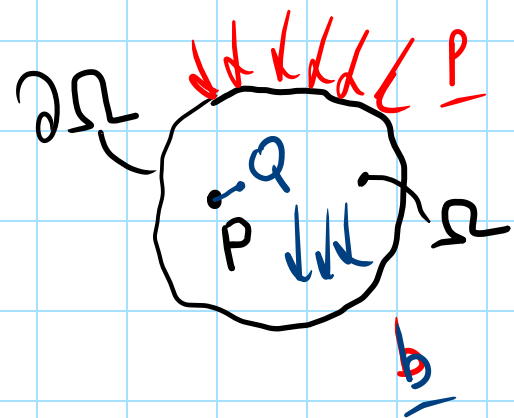
$$\underline{\underline{\sigma}} = \sigma_m \underline{\underline{I}} + \underline{\underline{\sigma}}^{\text{DEV}} \quad ; \quad \sigma_m = \frac{\text{tr} \underline{\underline{\sigma}}}{3} \quad ; \quad \text{"HYDROSTATIC PRESSURE"} \in \mathbb{R}$$

$$\underline{\underline{\sigma}}^{\text{DEV}} = \underline{\underline{\sigma}} - \sigma_m \underline{\underline{I}} \quad ; \quad \text{tr} \underline{\underline{\sigma}}^{\text{DEV}} = 0 \quad ; \quad \text{tr} \underline{\underline{\sigma}}^{\text{DEV}} = \text{tr} \underline{\underline{\sigma}} - \sigma_m 3 = \text{tr} \underline{\underline{\sigma}} - \text{tr} \underline{\underline{\sigma}} = 0$$

THIS DECOMPOSITION OF $\underline{\underline{\sigma}}$ IS FUNDAMENTAL TO FORMULATE FAILURE CRITERIA FOR MOST OF THE MATERIALS.

$$\begin{aligned} \text{---} \underline{\underline{\sigma}} &= \begin{bmatrix} 3 & 9 & 1 \\ 9 & 5 & 0 \\ 1 & 0 & -10 \end{bmatrix} \text{ MPa} \Rightarrow \sigma_m = -\frac{2}{3} \text{ MPa}, \quad [\underline{\underline{\sigma}}^{\text{DEV}}] = \begin{bmatrix} 3 - (-\frac{2}{3}) & 9 & 1 \\ 9 & 5 - (-\frac{2}{3}) & 0 \\ 1 & 0 & -10 - (-\frac{2}{3}) \end{bmatrix} \\ &= \begin{bmatrix} 11/3 & & \\ & 17/3 & \\ & & -28/3 \end{bmatrix} \end{aligned}$$

BALANCE EQUATIONS (EQUILIBRIUM, CHANGE IN $\underline{\underline{\sigma}}$ AT A CHANGE OF THE POINT)

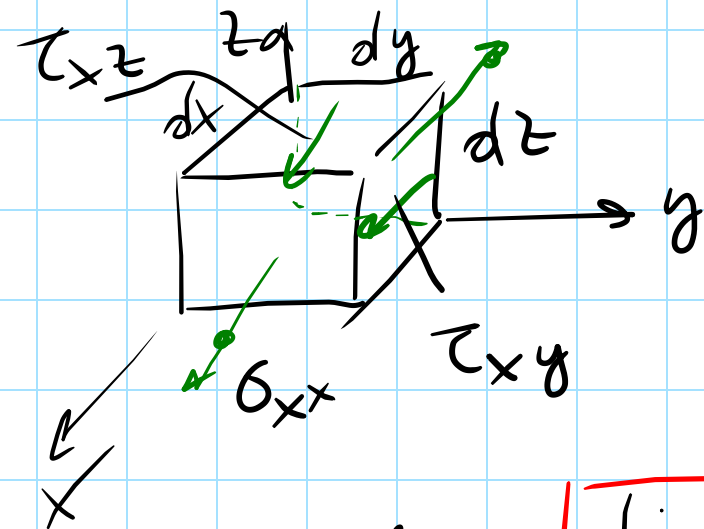


$$\underline{\underline{\sigma}}(P)$$

$$\underline{\underline{\sigma}}(Q) = \underline{\underline{\sigma}}(P) + d\underline{\underline{\sigma}}$$

$$\underline{\underline{t}} = \underline{\underline{\sigma}} \underline{\underline{m}}$$

SKETCH OF A PROOF:

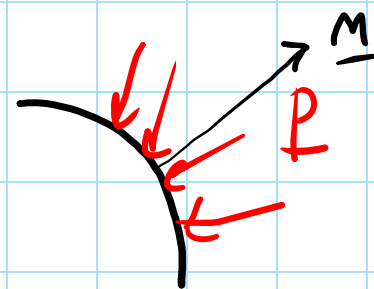


3 SCALAR BALANCE
EQS ALONG THE
3 AXES.

$$\begin{cases} \sigma_{xx,x} + \tau_{xy,y} + \tau_{xz,z} + b_x = 0 \\ \tau_{yx,x} + \sigma_{yy,y} + \tau_{yz,z} + b_y = 0 \\ \tau_{zx,x} + \tau_{zy,y} + \sigma_{zz,z} + b_z = 0 \end{cases}$$

OR $\boxed{\text{div } \underline{\underline{\sigma}} + \underline{\underline{b}} = \underline{\underline{0}}}$ in Ω GIVEN

WHAT ABOUT B.C.'S?



$\boxed{\underline{\underline{\sigma}} \underline{\underline{m}} = \underline{\underline{P}}}$ on $\partial\Omega$ UNKNOWN GIVEN

$$\begin{cases} \text{div } \underline{\underline{\sigma}} + \underline{\underline{b}} = \underline{\underline{0}} & (\Omega) \\ \underline{\underline{\sigma}} \underline{\underline{m}} = \underline{\underline{P}} & (\partial\Omega) \end{cases}$$

$$(\underline{\underline{\sigma}} = \underline{\underline{\sigma}}^T)$$

(RESULT THAT $\underline{\underline{\sigma}} = \underline{\underline{\sigma}}^T$ FROM BALANCE OF MOMENTA)

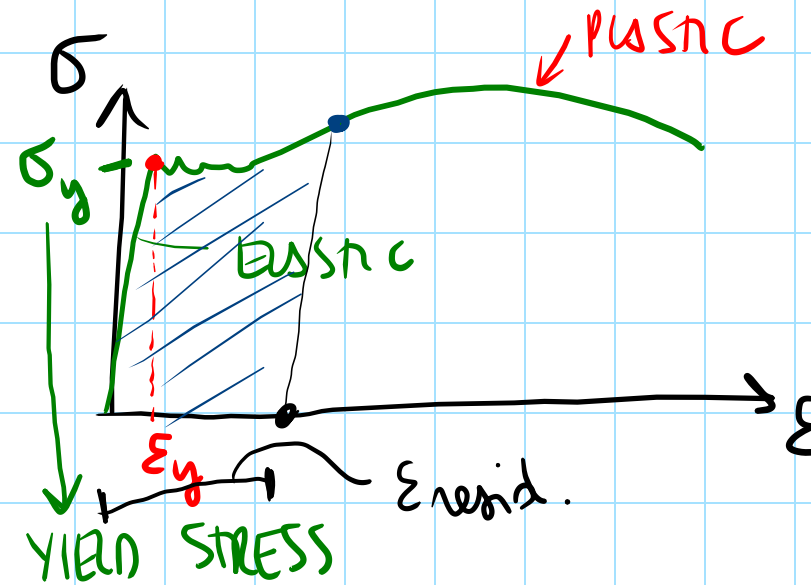
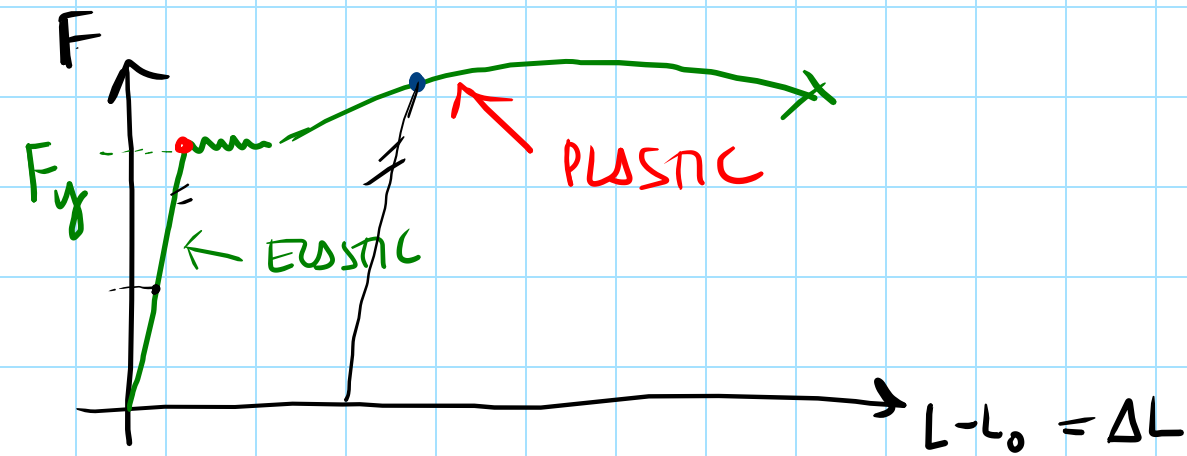
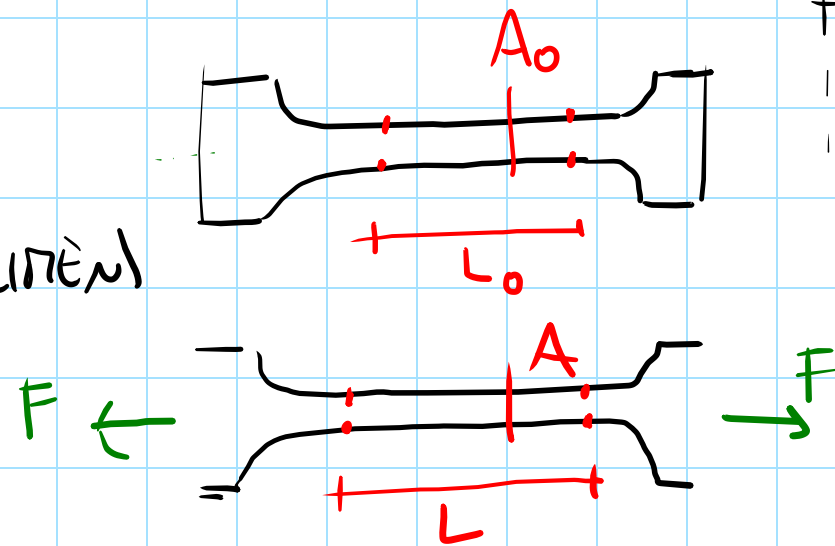
KINEMATICS : STRAIN, DEFORMATION

CONSTRUCTION STEEL

$$\sigma_y \approx 300 \text{ MPa}$$

$$\epsilon_y \approx 2\%$$

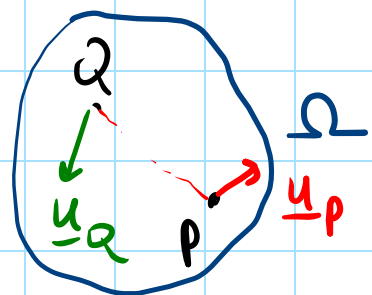
UNIAXIAL
TEST.
(STEEL SPECIMEN)



F_y : FORCE AT YIELDING

$$\sigma = \frac{F}{A} \approx \frac{F}{A_0}$$

$$\epsilon = \frac{\Delta L}{L_0} : \text{LONGITUDINAL STRAIN [-]}$$

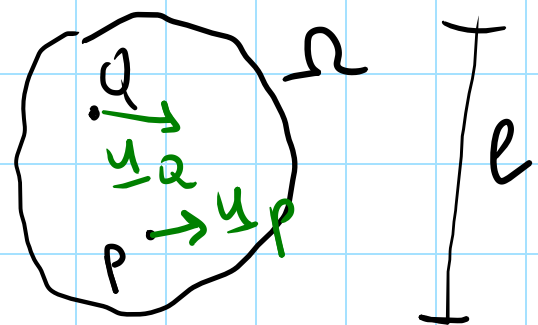


RIGID-BODY MOTION

$$\underline{u}_Q = \underline{u}_P + \underline{\omega} \times \underline{PQ}$$

$$\underline{\omega} = -\underline{\omega}^T : \text{ANTISYMMETRIC}$$

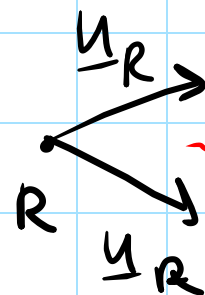
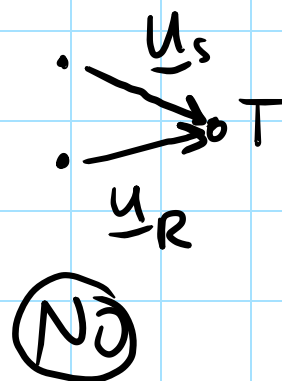
HYPOTHESES OF ANALYSIS OF STRAIN



- SMALL DISPLACEMENTS $|\underline{u}_P| \ll l \quad \forall P$

- SMALL DISPLACEMENT GRADIENTS $|\nabla \underline{u}_P| \ll 1 \quad \forall P$

S
R IN THE
NEIGHBORHOOD...

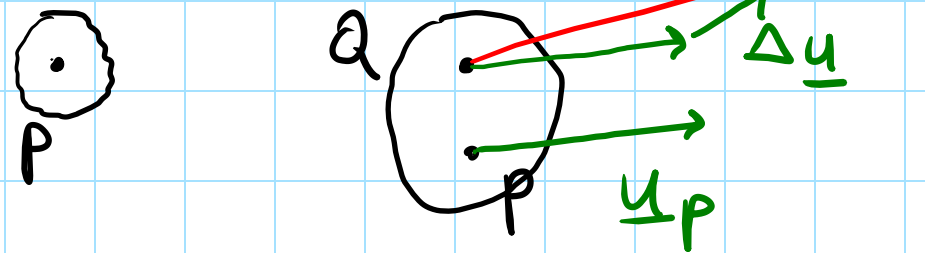


CREATION OF A
CRACK

NO

STRAIN AT A POINT $P \in \Omega$

NEIGHB... OF P



$$\underline{u}_Q = \underline{u}_P + \Delta \underline{u} \quad \text{TAYLOR EXPANSION}$$

$$= \underline{u}_P + \underbrace{\nabla \underline{u}(P) PQ}_{\text{LINEAR TERM}} + o(|PQ|)$$

$\underline{u}_P$ IS A DISPLACEMENT FIELD

SMALL -

$\underline{\underline{\varepsilon}}$: STRAIN TENSOR
($\underline{\underline{\varepsilon}} = \underline{\underline{\varepsilon}}^T$)

$$\underline{u}_Q - \underline{u}_P = \underline{\underline{\varepsilon}} PQ \quad (\text{RIGID-BODY MOTIONS APART})$$

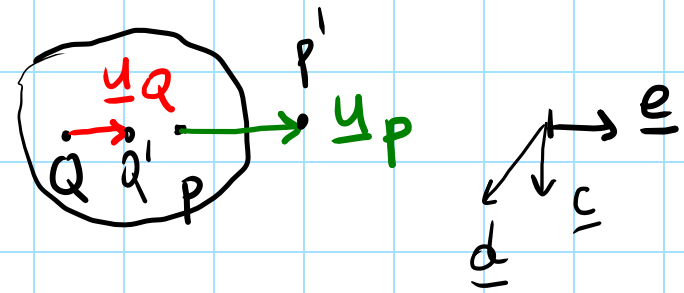
$$\underline{u}_Q \simeq \underline{u}_P + \nabla \underline{u}_P PQ = \underline{u}_P + (\underline{\underline{\varepsilon}} + \underline{\underline{w}}) PQ$$

$$= \underline{u}_P + \underbrace{\underline{\underline{\varepsilon}} PQ}_{\text{STRAIN}} + \underbrace{\underline{\underline{w}} PQ}_{\text{RIGID BODY}}$$

$$\varepsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i})$$

$$\begin{aligned} \varepsilon_{11} &= u_{1,1} \\ \varepsilon_{22} &= u_{2,2} \\ \varepsilon_{33} &= u_{3,3} \\ \varepsilon_{13} &= \frac{1}{2} (u_{1,3} + u_{3,1}) \end{aligned}$$

MEANING OF THE COMPONENTS OF $\underline{\underline{\epsilon}}$



$\{\underline{e}, \underline{c}, \underline{d}\}$ ORTHONORMAL BASIS

$$\underline{\underline{\epsilon}} = \epsilon_{11} \underline{e} \otimes \underline{e} + \epsilon_{12} \underline{e} \otimes \underline{c} + \epsilon_{13} \underline{e} \otimes \underline{d} \dots$$

$$\underline{u}_P = 0.5 \underline{e}$$

$$\underline{u}_Q = 0.2 \underline{e}$$

$$PQ = -20 \underline{e}$$

$$\underline{u}_Q - \underline{u}_P = \underline{\underline{\epsilon}} PQ$$

$$\underline{e}(0.2 - 0.5) = \underline{\underline{\epsilon}} \underline{e} (-20)$$

$$\underline{e}(-0.3) = \epsilon_{11} \underline{e} (-20)$$

$$\epsilon_{11} = \frac{-0.3}{-20} = +0.015$$

$\epsilon_{11} > 0$: ELONGATION ALONG DIRECTION 1

IN $[\underline{\underline{\epsilon}}]$, DIAGONAL TERMS ARE THE DISTORTIONS ALONG THE AXIS OR VECTORS OF THE ORTHONORMAL BASIS.

$\epsilon_{11} < 0$: CONTRACTION ALONG DIRECTION 1

$$|PQ| = L_0$$

$$|P'Q'| = L$$

$$|P'Q'| = 20 + 0.5 - 0.2 = 20.3$$

$$\epsilon = \frac{\Delta L}{L_0} = \frac{0.3}{20} = +0.015$$