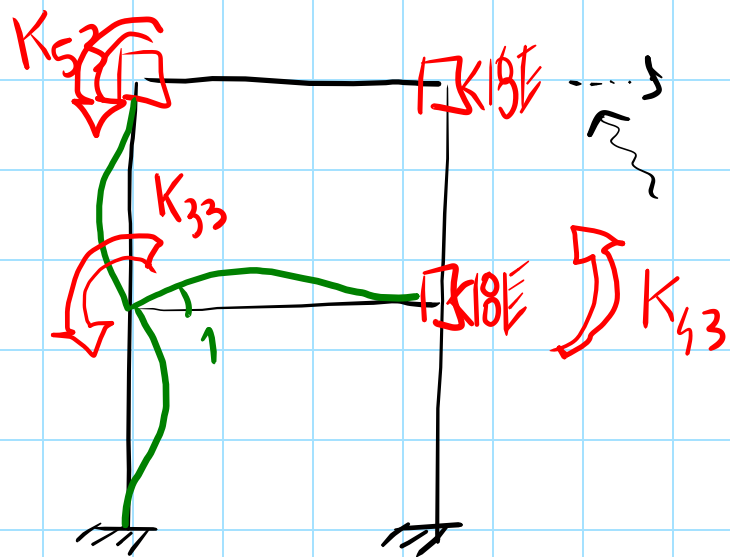


... CONTINUE THE EX

$q_3 = 1, q_1 = q_2 = q_4 = \dots q_6 = 0$ (3rd column of $\underline{K}$)



$$K_{33} = 2 \cdot \frac{4EI}{h} + \frac{4EI}{e}$$

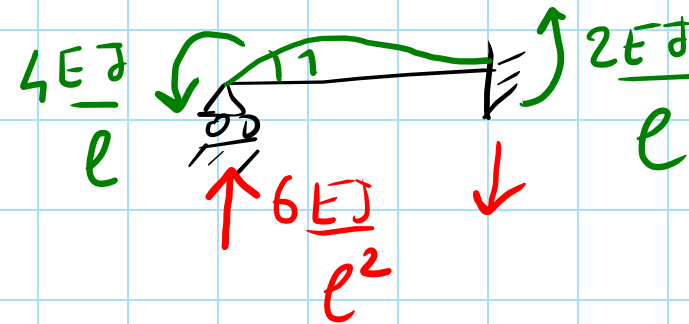
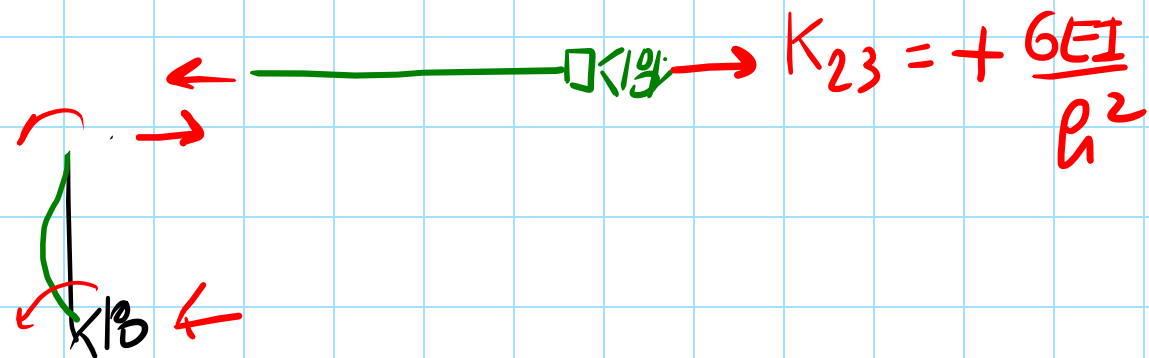
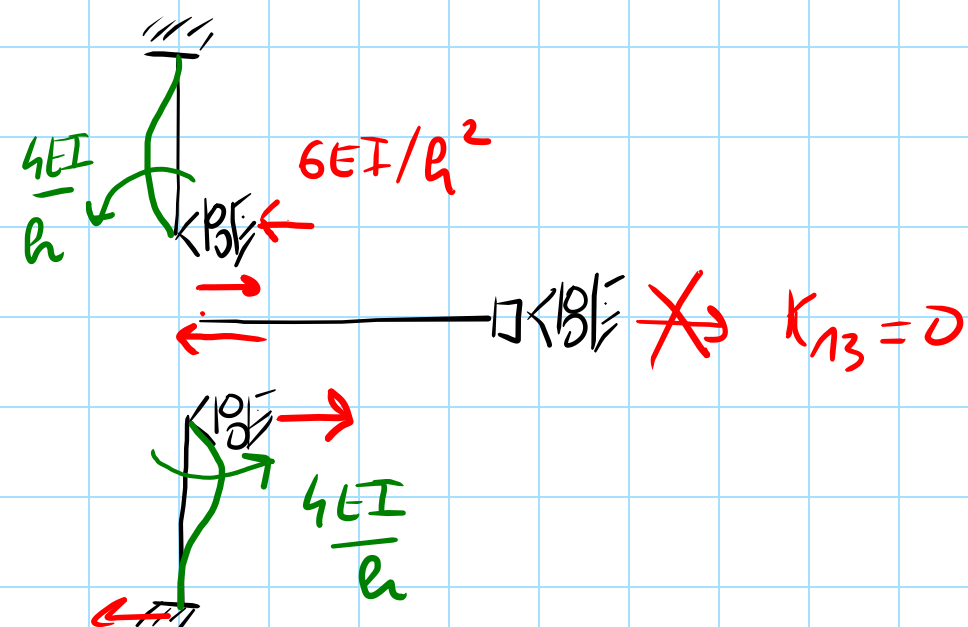
$$K_{43} = + \frac{2EI}{e}$$

$$K_{53} = + \frac{2EI}{h}$$

$$K_{63} = 0$$

K_{13}

K_{23}



$$\underline{K} = \begin{bmatrix} \vdots \\ \vdots \\ \vdots \end{bmatrix}$$

SYM

$$\begin{bmatrix} 0 & +\frac{6EI}{h^2} & \frac{8EI}{h} + \frac{4EI}{e} \\ +\frac{6EI}{h^2} & \frac{4EI}{e} & \frac{2EI}{e} \\ \frac{8EI}{h} + \frac{4EI}{e} & \frac{2EI}{e} & 0 \end{bmatrix}$$

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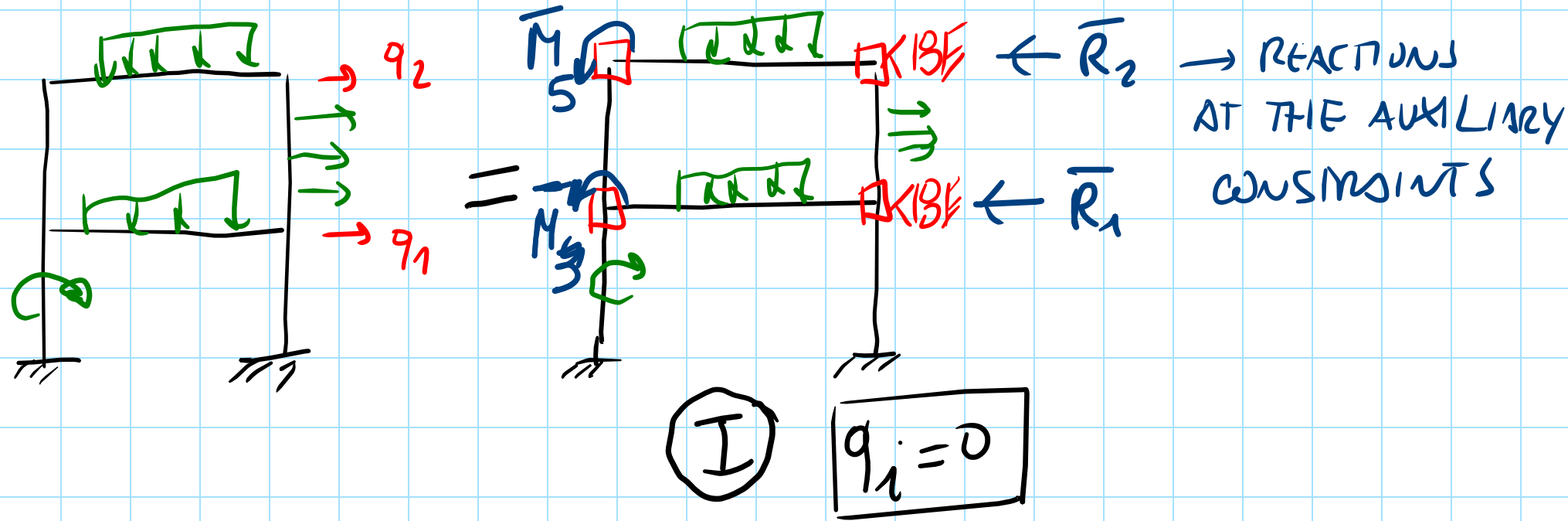
NOTE: DIMENSIONS OF K_{ij} DEPEND ON THE LINEAR SYSTEM:

$$\begin{bmatrix} \vdots \\ \vdots \\ \vdots \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \\ \hline q_3 \\ q_4 \\ \vdots \end{bmatrix} = \begin{bmatrix} F_1 \\ F_2 \\ \hline m_3 \\ m_4 \\ \vdots \end{bmatrix}$$

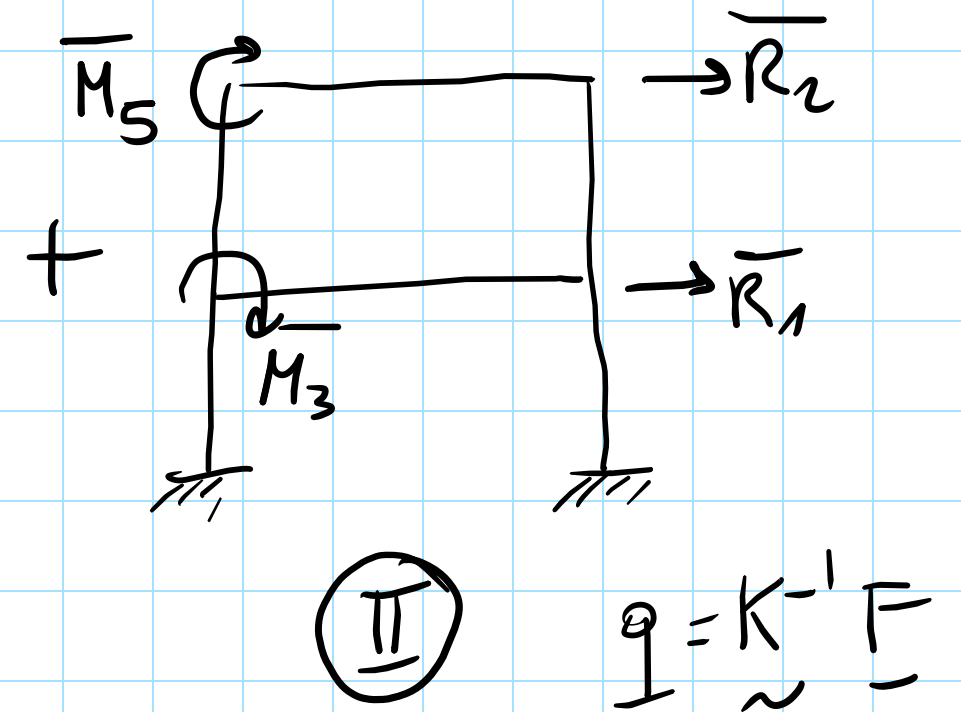
DISPL
FORCE
ROTATION
MOMENT

$$\begin{aligned} K_{11}q_1 + K_{12}q_2 + K_{13}q_3 + \dots &= \text{FORCE} \\ \dots &\dots \\ K_{31}q_1 + K_{32}q_2 + K_{33}q_3 + \dots &= \text{MOMENT} \end{aligned}$$

IN THE EXAMPLE ABOVE THE SYSTEM $\underline{\tilde{K}}\underline{q} = \underline{F}$ IS THE "SCHEME II" IN THE DISPLACEMENT METHOD

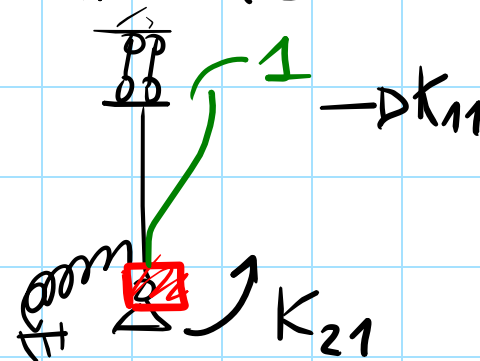
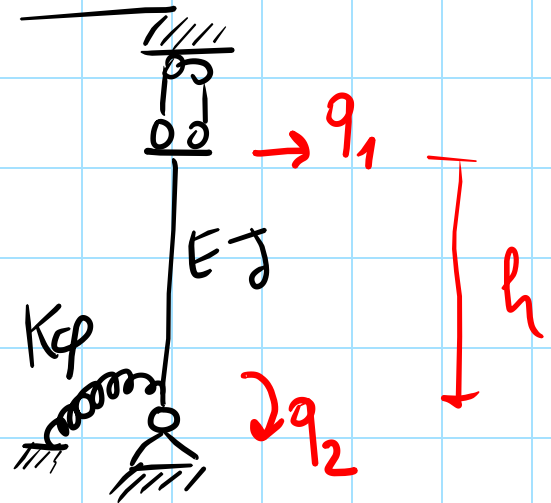


K_{13} : FORCE
 K_{31} : FORCE

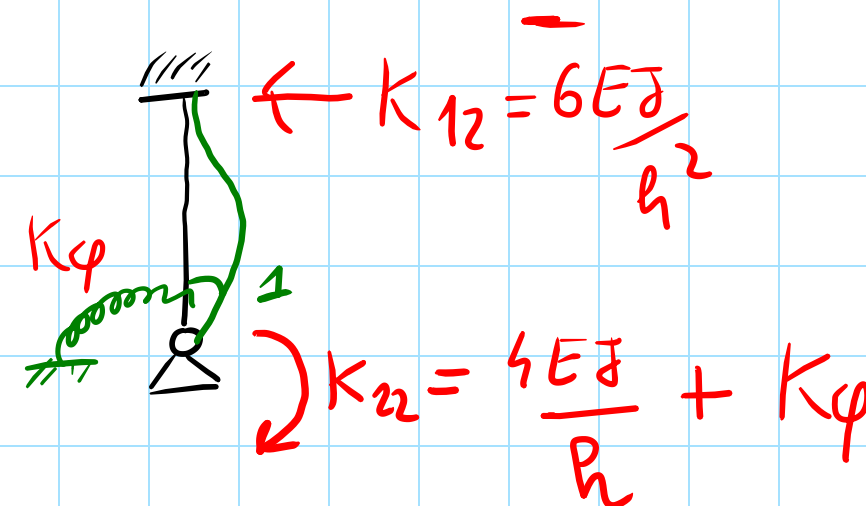


NOTE: EFFECT OF COMPLIANT CONSTRAINTS

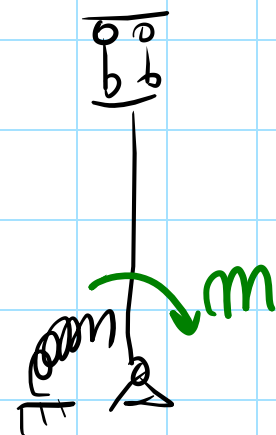
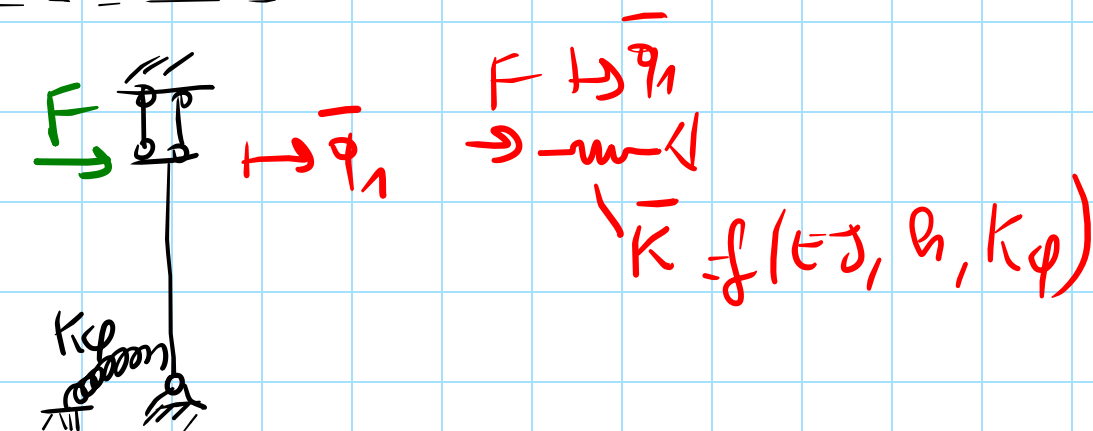
$$q_1 = 1, q_2 = 0$$



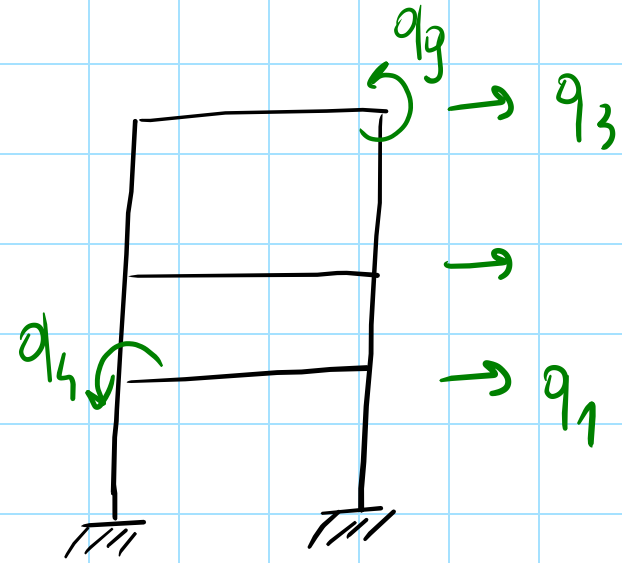
$$q_1 = 0, q_2 = 1$$



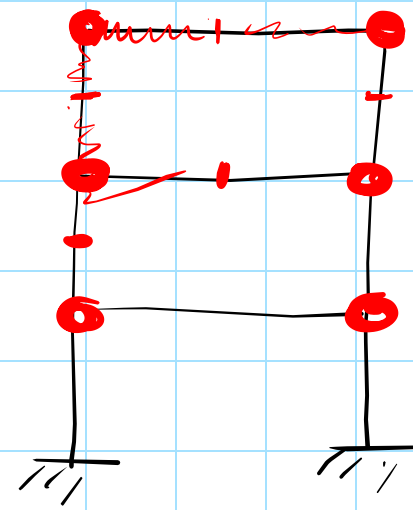
$$K_z = \begin{bmatrix} 12EI/h^3 & -6EI/h^2 \\ -6EI/h^2 & 4EI/h + K_\phi \end{bmatrix}$$



CONDENSATION OF STIFFNESS MATRIX

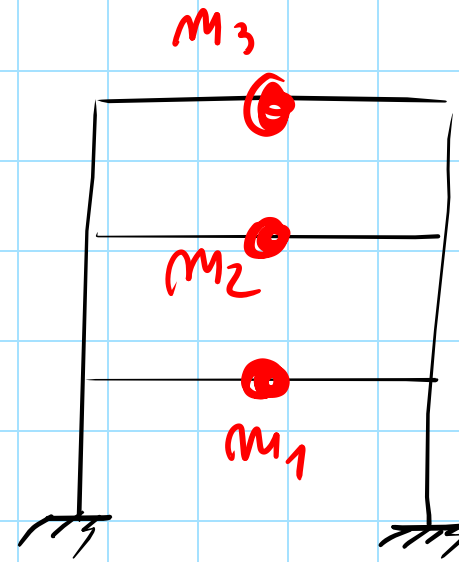


MASS
MATRIX



CONCENTRATED
MASSES

=



$$\underline{q}_A = \begin{bmatrix} q_1 \\ q_2 \\ q_3 \end{bmatrix}; \quad \underline{q}_B = \begin{bmatrix} q_4 \\ \vdots \\ q_9 \end{bmatrix}$$

$$\underline{\tilde{M}} \ddot{\underline{q}} + \underline{\tilde{K}} \underline{q} = \underline{\tilde{F}}(t)$$

$$\begin{bmatrix} \underline{\tilde{M}}_A & \underline{\tilde{O}} \\ \underline{\tilde{O}} & \underline{\tilde{O}} \end{bmatrix} \begin{bmatrix} \underline{q}_A \\ \underline{q}_B \end{bmatrix} + \begin{bmatrix} \underline{\tilde{K}}_{AA} & \underline{\tilde{K}}_{AB} \\ \underline{\tilde{K}}_{BA} & \underline{\tilde{K}}_{BB} \end{bmatrix} \begin{bmatrix} \underline{q}_A \\ \underline{q}_B \end{bmatrix} = \begin{bmatrix} \underline{\tilde{F}}_A(t) \\ \underline{\tilde{O}} \end{bmatrix}$$

$$\underline{\tilde{M}}_A = \begin{bmatrix} m_1 & 0 & 0 \\ 0 & m_2 & 0 \\ 0 & 0 & m_3 \end{bmatrix}$$

WE CAN SHOW THAT THE SYSTEM
CAN BE SOLVED ONLY IN
TERM OF VECTOR $\underline{q}_A$

$$\underline{\tilde{K}}_{BA} \underline{q}_A + \underline{\tilde{K}}_{BB} \underline{q}_B = \underline{0} \quad ; \quad \underline{q}_B = - \underline{\tilde{K}}_{BB}^{-1} \underline{\tilde{K}}_{BA} \underline{q}_A \quad (*) \quad (\text{BOTTOM SUB SYSTEM})$$

$$\underline{M}_{\tilde{A}} \ddot{\underline{q}}_A + \underline{\tilde{K}}_{AA} \underline{q}_A + \underline{\tilde{K}}_{AB} \underline{q}_B = \underline{F}_A(t) \quad \Rightarrow \quad \underline{M}_{\tilde{A}} \ddot{\underline{q}}_A + \underbrace{\left[\underline{\tilde{K}}_{AA} - \underline{\tilde{K}}_{AB} \underline{\tilde{K}}_{BB}^{-1} \underline{\tilde{K}}_{BA} \right]}_{\text{CONDENSED STIFFNESS MATRIX}} \underline{q}_A = \underline{F}_A(t)$$

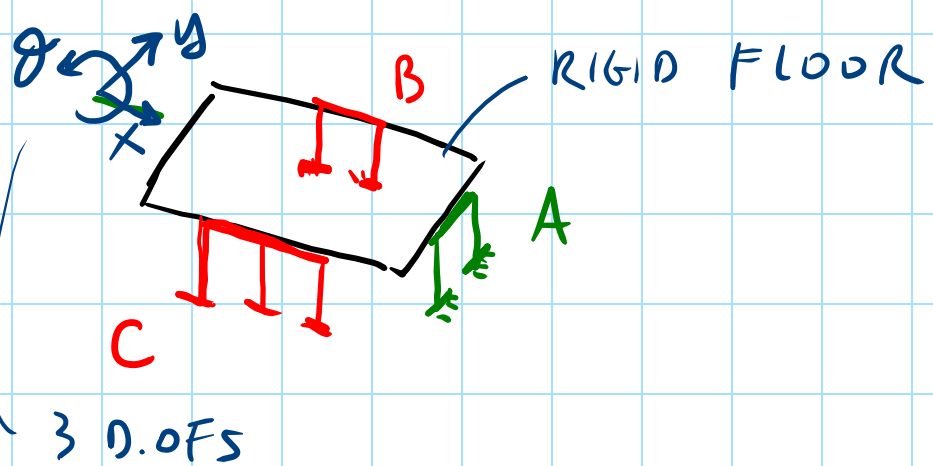
(TOP SUB SYSTEM)

CONDENSED STIFFNESS MATRIX

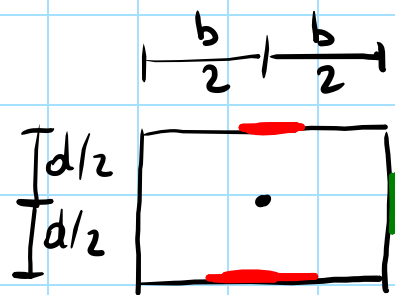
$\Rightarrow$ COMPUTE $\underline{q}_A(t)$

THROUGH $(*)$ $\underline{q}_B$ CAN BE CALCULATED.

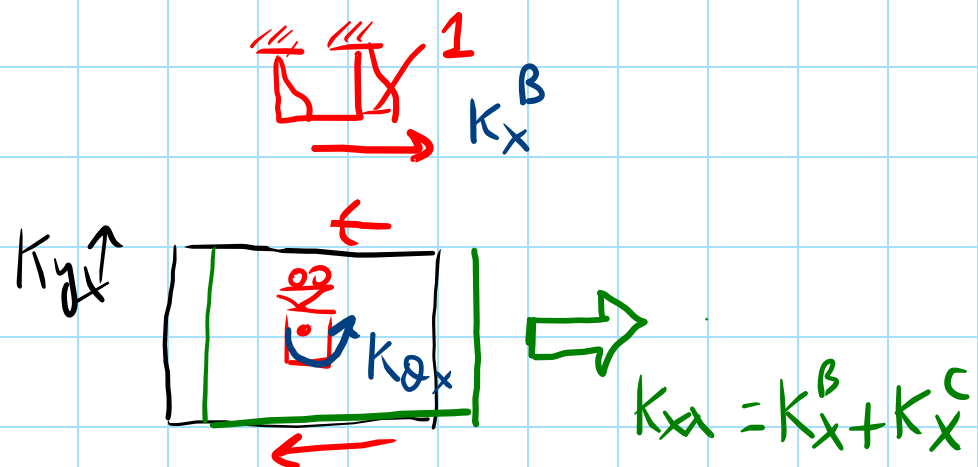
EXAMPLE OF COMPUTAT. OF A STIFFNESS MATRIX IN 3D



$$\boxed{x=1, y=\theta=0}$$



NEGLECT RESISTANCE
OF FRAMES
OUT OF PLANE

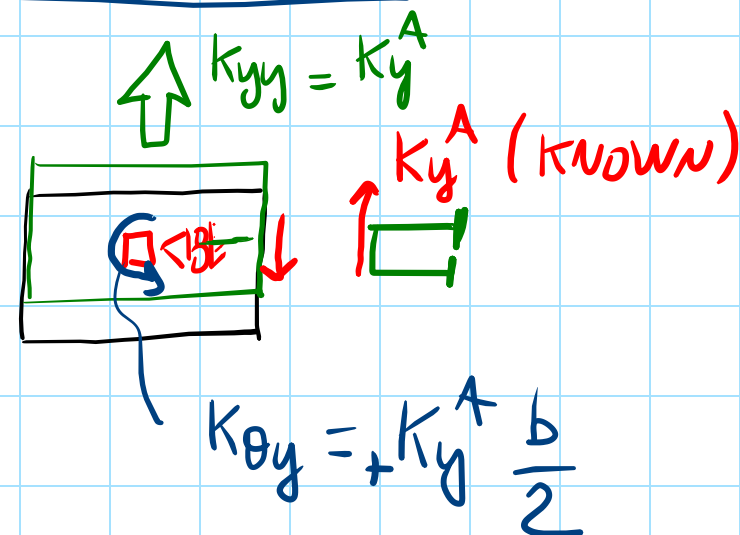


1  $K_X^C (> K_X^B)$

$$K_{yx} = 0$$

$$K_{\theta x} ? = + K_x^c \frac{d}{2} - K_x^B \frac{d}{2} = (K_x^c - K_x^B) \frac{d}{2}$$

$$y=1, x=0 \Rightarrow$$



$$K_{xy} = 0$$