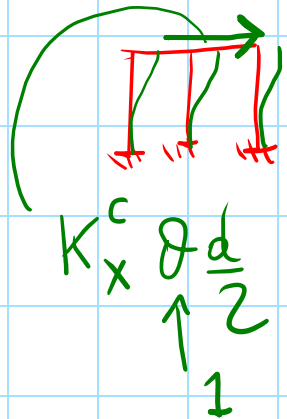
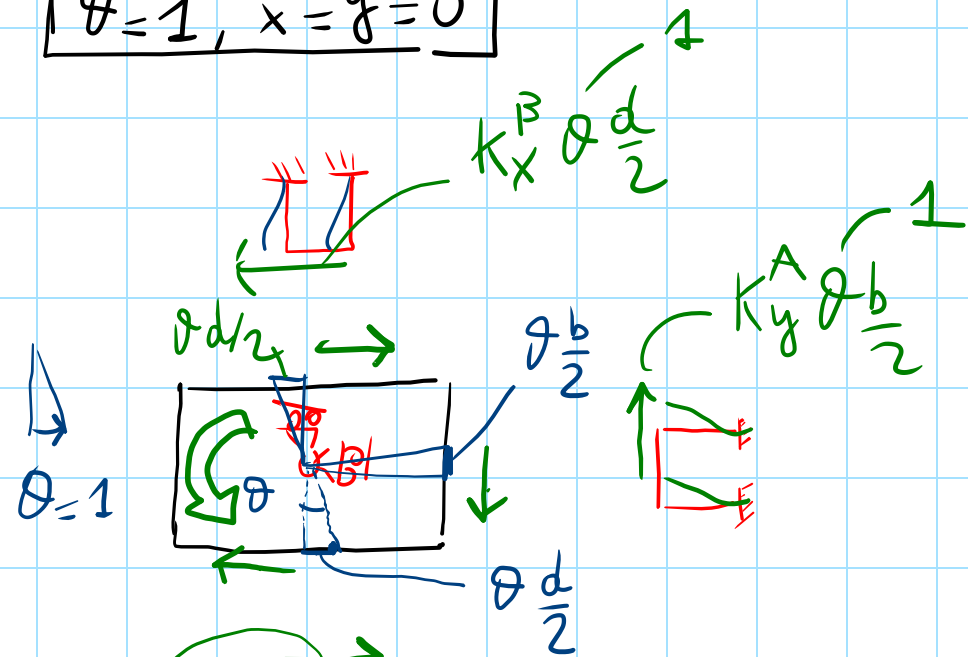


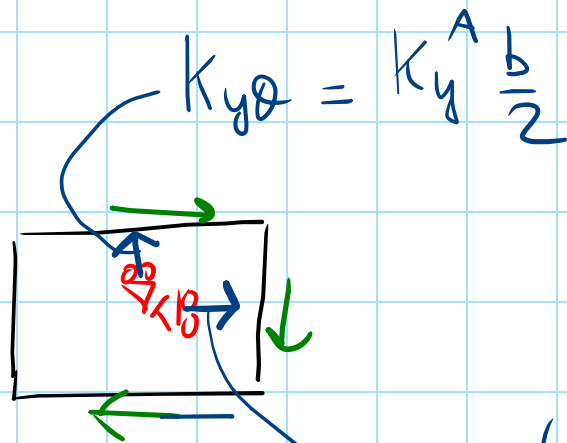
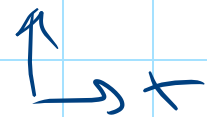
$$\theta = 1, x = y = 0$$



$$K_{\theta\theta} = K_x^C \frac{d}{2} + K_y^A \frac{b}{2} + K_x^B \frac{d}{2} \frac{d}{2}$$

$$K_{x\theta} =$$

RECALL, SYMMETRY IS DUE TO THE BETTI'S THEOREM



$$K_{x\theta} = (K_x^C - K_x^B) \frac{d}{2}$$

$$[K] = \begin{bmatrix} K_x^C + K_x^B & 0 & (K_x^C - K_x^B) \frac{d}{2} \\ 0 & K_y^A & K_y^A \frac{b}{2} \\ (K_x^C - K_x^B) \frac{d}{2} & K_y^A \frac{b}{2} & K_{\theta\theta} \end{bmatrix}$$

$$[K] = \begin{bmatrix} x & 0 & 0 \\ 0 & x & 0 \\ 0 & 0 & x \end{bmatrix}$$

$K_{\sim}$ DIAGONAL

14/5/25

1) FRAME B = FRAME C (x: SYMM.)

$$[K] = \begin{bmatrix} x & 0 & 0 \\ 0 & x & x \\ 0 & x & x \end{bmatrix}$$

2) $\rightarrow x$

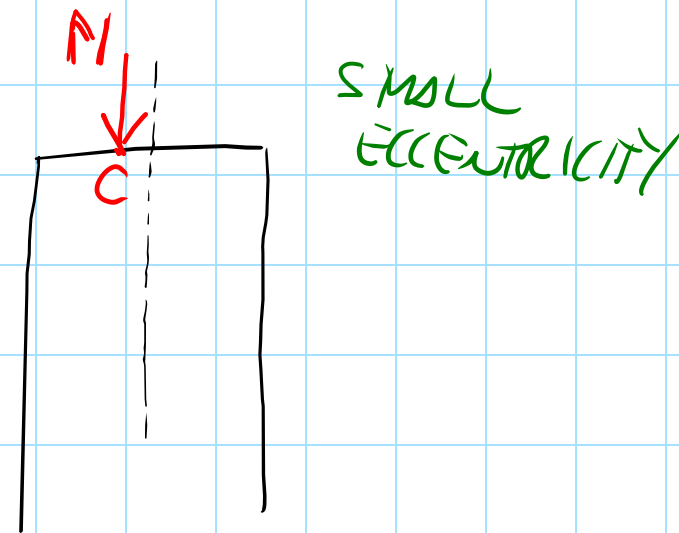
x, y: AXES OF SYMMETRY

MECHANICS OF MASONRY STRUCTURES (COMPRESSED PILLS, WALLS, ARCHES)

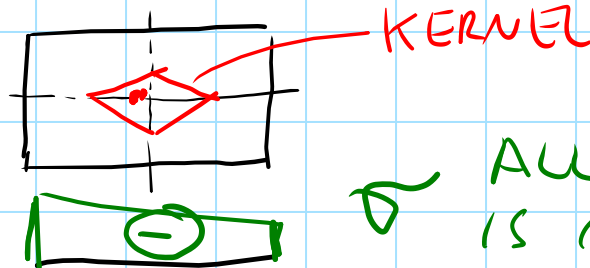
HEYMAN'S HYPOTHESES FOR MODELLING OF MASONRY STRUCTURES

- 1) MASONRY HAS NO TENSILE RESISTANCE
- 2) " " INFINITE COMPRESSION RESISTANCE
- 3) FAILURE OF A STRUCTURE DUE TO SLIDING OF BLOCKS IS RULED OUT (FRICTION WILL SAVE YOU)

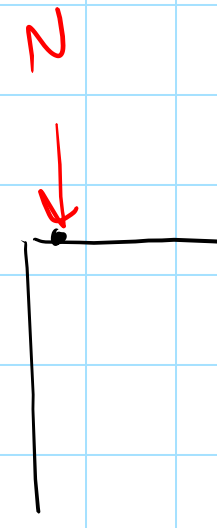
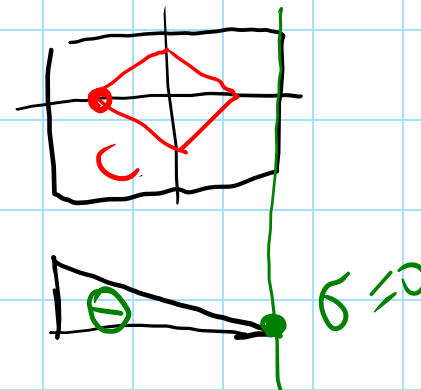
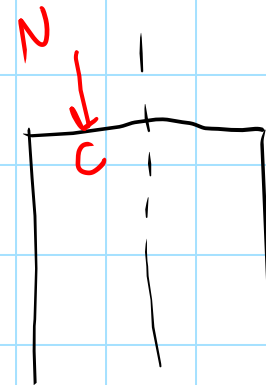
COMPRESSED PILLS: NO-TENSION MATERIAL



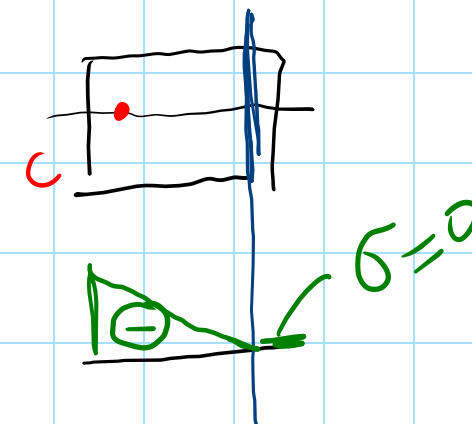
SMALL
ECCENTRICITY



ALL SECTION
IS COMPRESS.

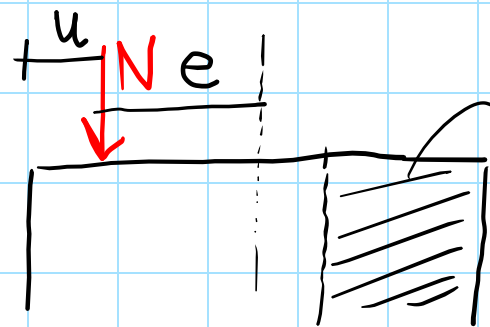


LARGE
ECCENTRICITY



NAVIER SOL.
NOT VALID!

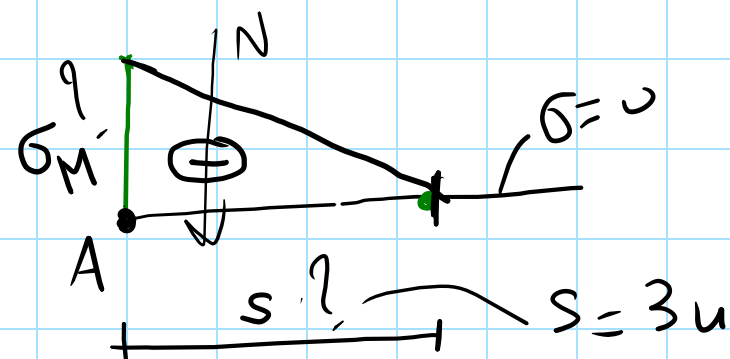
COMPRESSION OF A NO-TENSION PILLAR LOADED AT "LARGE ECCENTRICITY"



FAILURE / CRACKS /

$$\begin{cases} \sigma_M s b \frac{1}{2} = N \\ N u = \underbrace{\sigma_M s b \frac{1}{2}}_N \cdot \frac{s}{3} \end{cases} \rightarrow \sigma_M 3 u b \frac{1}{2} = N ; \boxed{\sigma_M = \frac{2N}{3ub}} \quad (\text{EQUIV. MOM W.R.T. A})$$

$$\rightarrow u = \frac{s}{3}$$



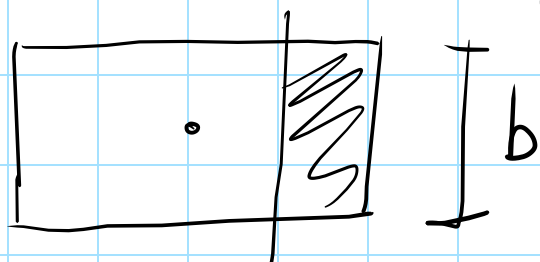
WHAT IF MASONRY HAS GOT A LIMITED RESISTANCE IN COMPRESSION? f_k : MAX STRESS IN COMPRESSION

$$u \rightarrow u_{\min} ! \quad u_{\min} = \frac{2N}{3b f_k}$$

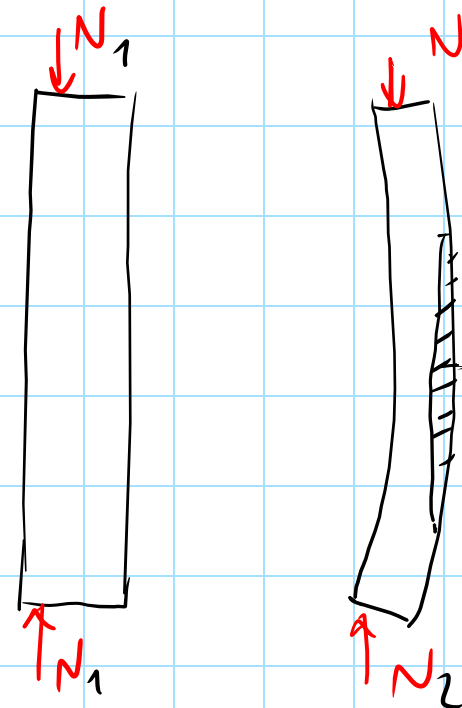
BY IMPOSING EQUIVALENCE OF FORCES AND MOMENTS

$$\Rightarrow \sigma_M s$$

CROSS SECTION

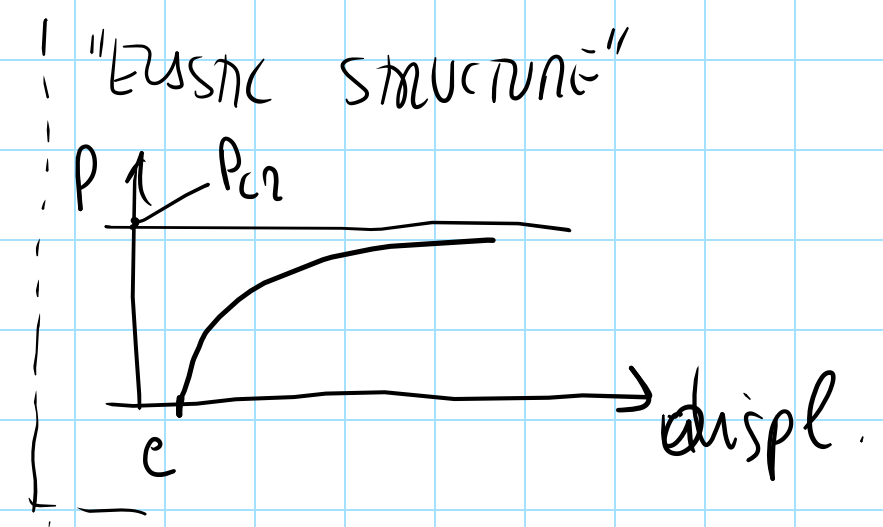


INSTABILITY OF A PILLAR LOAD AT "LARGE ECCENTRICITY"



AT WHICH
 $N_2 > N_1$
 DOES THE
 PILLAR BECOME
 UNSTABLE?
 (FAILURE)

YOKEL
 SALIN 60s
 FRISCH FAY 70s



DEVELOP A "ELASTIC LINE" APPROACH
 AND STUDY PROGRESSIVE INSTABILITY.