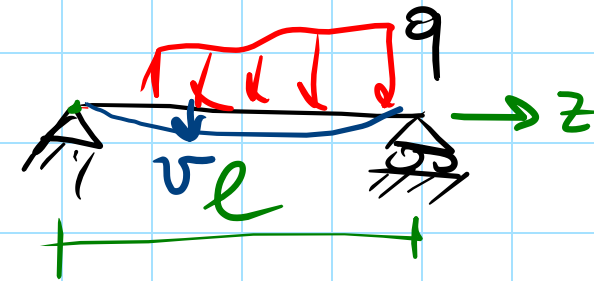


STRONG AND WEAK FORMULATIONS OF A DIFFERENTIAL PROBLEM CSM, 2/3/26

• THE PROBLEM OF THE "ELASTICA" :



$v(z)$: DISPLACEMENT MAP (ELASTIC LINE)

$$(EJ v'')'' = q$$

↑
UNKNOWN

(IV-ORDER DIFF EQ.)

+ 4 B. CONDITIONS

STRONG FORMULATION
OF THE 'ELASTIC LINE'
PROBLEM

WE CAN FORMULATE THE SAME PROBLEM THROUGH THE TH. OF VIRTUAL WORK :

$$\int_{STR} M K dz = \int_{SM} q^* v dz$$

↑
CURVATURE ($v'' = K$)

WEAK FORMULATION OF THE 'ELASTIC LINE PROBLEM'

ANOTHER NEW FORMULATION

IS THAT BASED ON THE TOTAL POTENTIAL ENERGY

COMPUTATIONAL MECHANICS (LINEAR ELASTICITY)

- COMPUTATIONAL MODEL $\xrightarrow{\text{AIM}}$
(APPROXIMATED WITH RESPECT TO
THE MATHEMATICAL ELASTIC PROBLEM)

$$\underset{\substack{\uparrow \\ \text{PROPERTY OF THE PROBLEM}}}{\tilde{K}} \underset{\substack{\uparrow \\ \text{UNKNOWN (FINITE NUMBER)}}}{u} = \underset{\substack{\leftarrow \\ \text{DATA}}}{F} \quad (\text{LINEAR SYSTEM})$$

SOURCES OF APPROXIMATION:

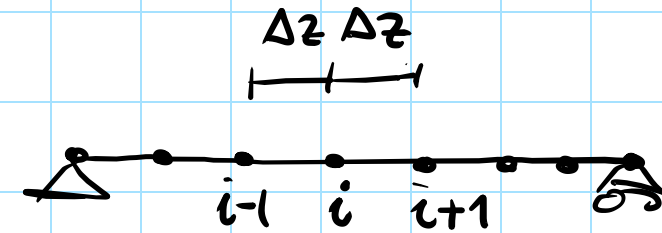
- 1) FROM THE DOMAIN (DISCR. N° OF NODES)
 - 2) FROM THE "INVOLVED FUNCTIONS" (DESCRIPTION OF DISPLACEMENT / DESCR. OF LOADS)
 - 3) FROM THE NUMERICAL METHOD
 - 4) ... LIMITED CAPABILITY OF THE CPU
- FROM THE PROBLEM
- COMPUTATIONAL RESOURCES

REGARDING THE "NUMERICAL METHOD"

1) FINITE DIFFERENCE METHOD (LATE '800)

$(EJ v'')'' = q \rightarrow$ "DISCRETIZE" THE DERIVATIVES

STRONG FORMULATION



$$\frac{\Delta v}{\Delta z} \approx \frac{v_{i+1} - v_i}{\Delta z} \quad \text{EXAMPLE OF DISCR OF } v'(z)$$

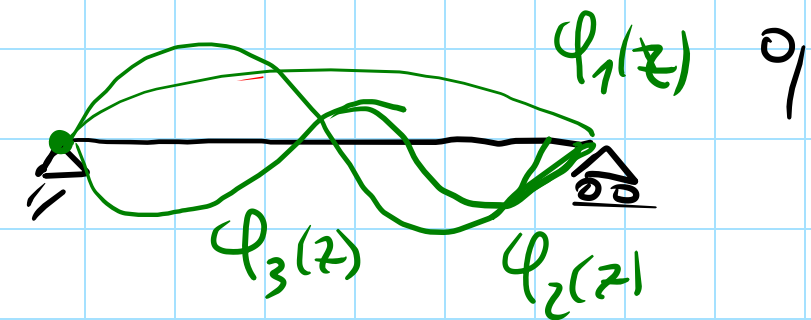
2) RAYLEIGH-RITZ METHOD (LATE '800)

WEAK FORMULATION

$$v(z) = \underbrace{d_1}_{\uparrow} \underbrace{\phi_1(z)}_{\uparrow} + \underbrace{d_2}_{\uparrow} \underbrace{\phi_2(z)}_{\uparrow} + \underbrace{d_3}_{\uparrow} \underbrace{\phi_3(z)}_{\uparrow}$$

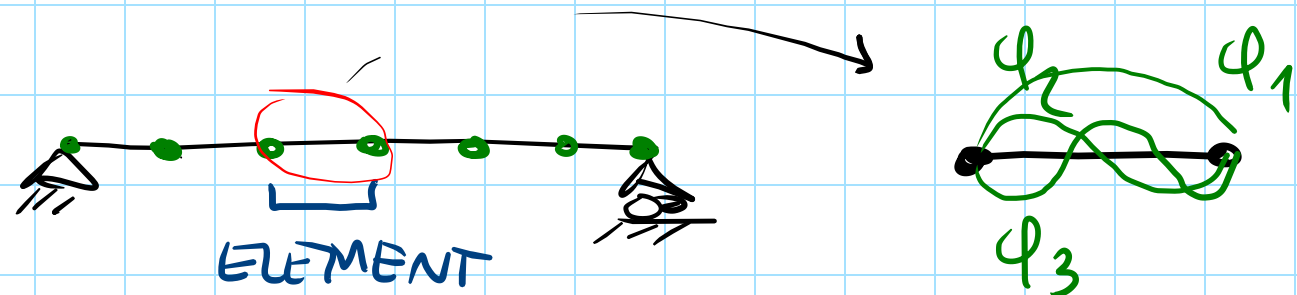
SUITABLE SET OF FUNCTIONS

UNKNOWN OF THE PROBLEM (FINITE NO) RITZ COEFFICIENTS



3) FINITE ELEMENT METHOD (FEM) (1920 GALERKIN, '50 CLOUGH, MILBERT...)

(extension of RAYLEIGH-RITZ METHOD)



THE DOMAIN IS DIVIDED IN DISCRETE ELEMENTS AND WITHIN EACH ELEMENT THE SOLUTION IS SOUGHT :

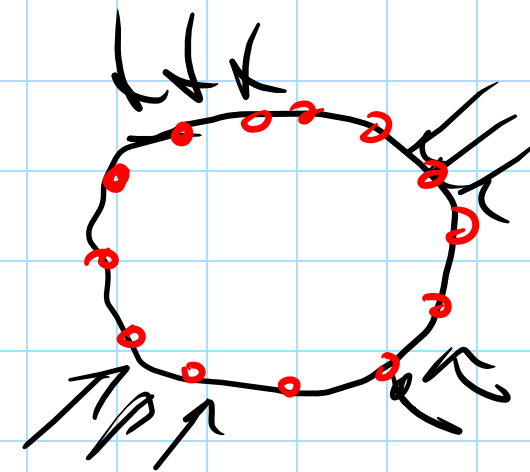
$$u(z) = \alpha_1 \varphi_1(z) + \alpha_2 \varphi_2(z) + \alpha_3 \varphi_3(z) \dots$$

RITZ COEFFIC.
FOR THAT ELEMENT.

THERE ARE TWO TYPES OF APPROXIMATION :

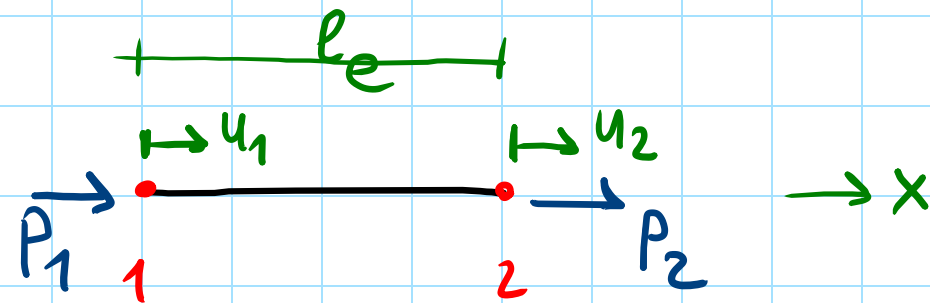
- GEOMETRIC (MESH)
- ANALYTICAL ($\varphi_1, \varphi_2, \varphi_3 \dots$)

4) BEM : BOUNDARY ELEMENT METHOD



FEM IN ELASTICITY OF BARS

- STIFFNESS MATRIX OF A BAR ($EA = \text{CONST}$)



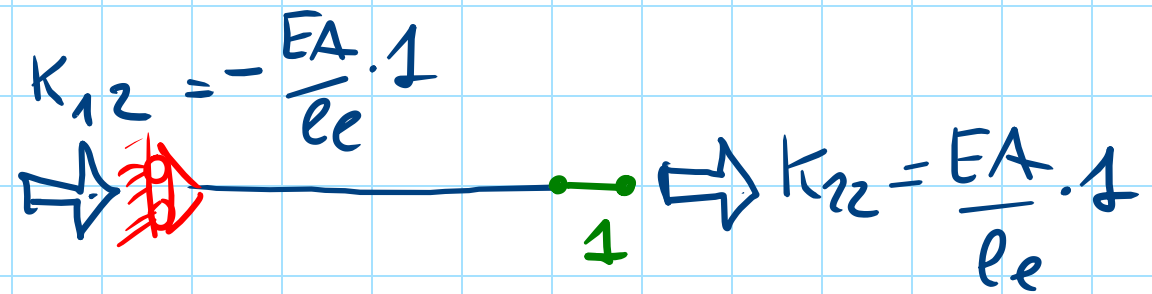
$$\begin{bmatrix} P_1 \\ P_2 \end{bmatrix} = \frac{EA}{l_e} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

\underline{K} (SYMMETRIC, SINGULAR)

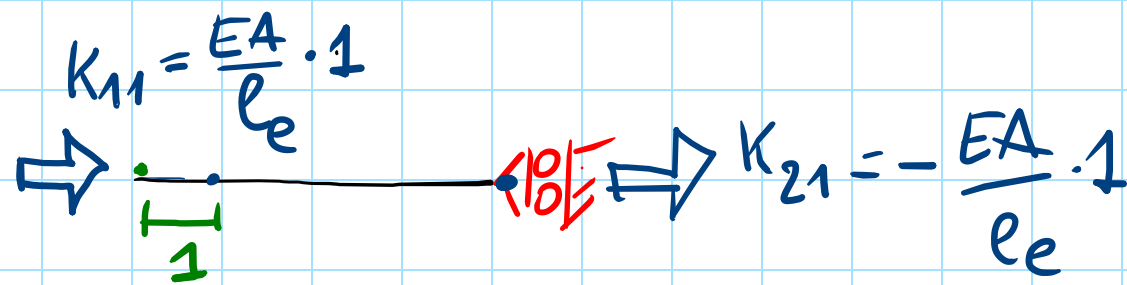
$$\begin{bmatrix} P_1 \\ P_2 \end{bmatrix} = \begin{bmatrix} K_{11} & K_{12} \\ K_{21} & K_{22} \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

K_{ij} : FORCE in i -th NODE WHEN $u_j = 1$ AND $u_i = 0$ ($i \neq j$)

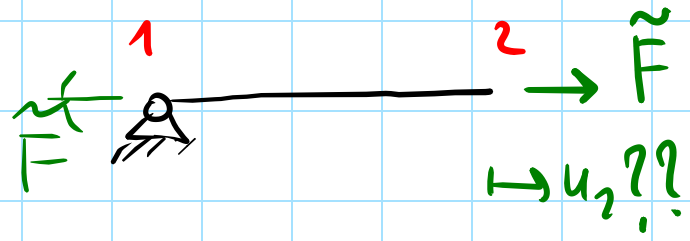
$$\left. \begin{matrix} u_1 = 0 \\ u_2 = 1 \end{matrix} \right\} \Rightarrow \begin{matrix} K_{12} \\ K_{22} \end{matrix}$$



$$\left. \begin{matrix} u_1 = 1 \\ u_2 = 0 \end{matrix} \right\} \Rightarrow \begin{matrix} K_{11} \\ K_{21} \end{matrix}$$



e structural



$$\begin{bmatrix} P_1 \\ \hat{F} \end{bmatrix} = \frac{EA}{l_e} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} 0 \\ u_2 \end{bmatrix}$$

$$\begin{cases} P_1 = \frac{EA}{l_e} (-u_2) \\ \hat{F} = \frac{EA}{l_e} u_2 \end{cases}$$

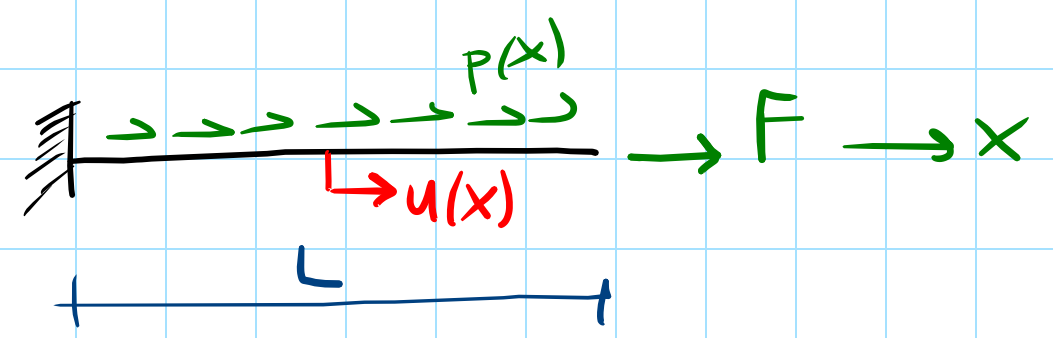
e) $P_1 = -\hat{F}$

1) $u_2 = \frac{\hat{F} l_e}{EA}$

(TWO-STEP SOLUTION)

LET US COME BACK TO OUR GOAL (FEM IN BARS)

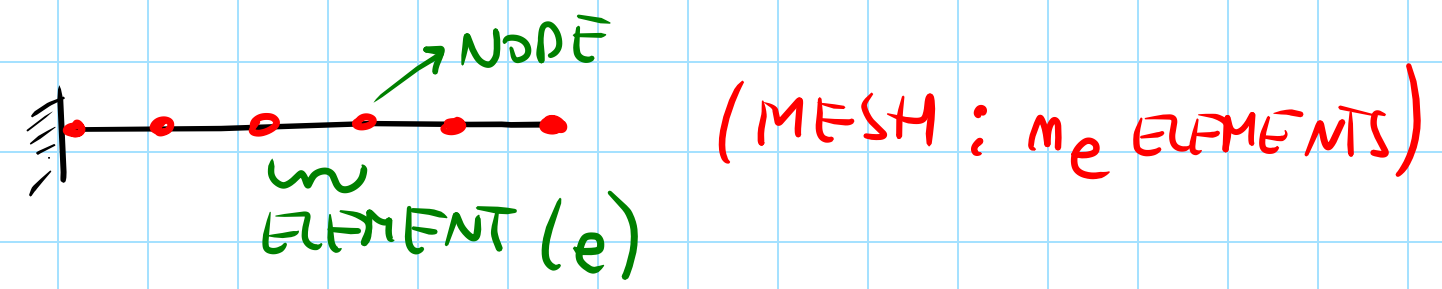
"FEM IN BARS"



WEAK FORMULATION \rightarrow STAT. OF TOT. POT. ENERGY

$$\Pi(u) = \frac{1}{2} \int_0^L EA (u')^2 dx - \int_0^L p u dx - F(0)u(0) - F(L)u(L)$$

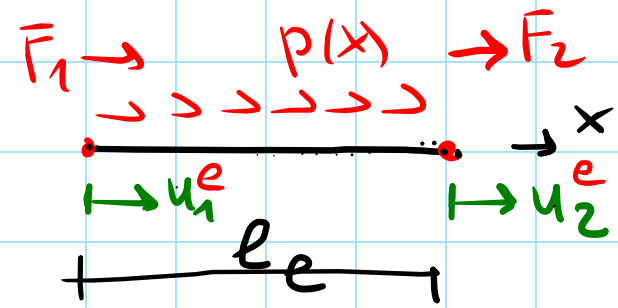
1) DISCRETIZATION



$$\Pi(u) \approx \sum_{e=1}^{m_e} \Pi^e(u^e(x))$$

?? \Rightarrow LET US STUDY EACH ELEMENT

2) APPROXIMATION OF THE DISPL. FIELD ($u_e(x)$) IN EACH ELEMENT



ELEMENT e ($EA: \text{CONST}$)

(NODAL DISPLACEMENTS)

$$u(x) = d_1 + d_2 x \quad (\text{LINEAR COMBINATION}) \quad x \in [0, l_e]$$

UNKNOWN(S) (RITZ COEFFS)

$$d_1, d_2 = f(u_1, u_2) ?$$

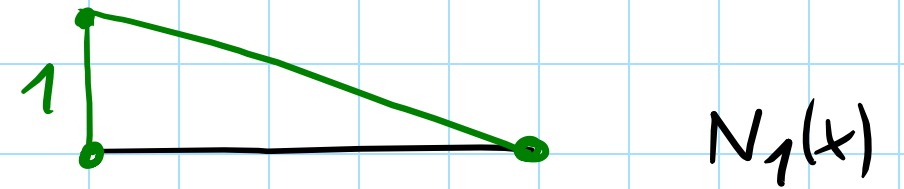
$$\begin{cases} u(0) = u_1 = d_1 \\ u(l_e) = u_2 = d_1 + d_2 l_e \end{cases}$$

$$\begin{cases} d_1 = u_1 \\ d_2 = \frac{u_2 - u_1}{l_e} \end{cases}$$

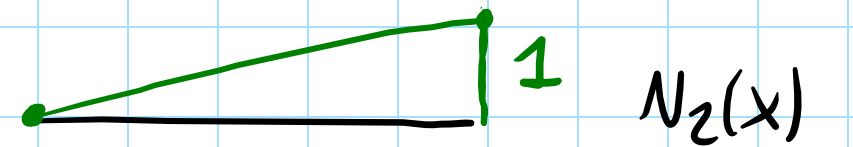
THUS

$$u(x) = u_1 + \frac{u_2 - u_1}{l_e} x = u_1 \left(1 - \frac{x}{l_e}\right) + u_2 \left(\frac{x}{l_e}\right) = \begin{bmatrix} N_1(x) & N_2(x) \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = \underline{N} \underline{U}$$

SCALAR PRODUCT



$N_1(x)$



$N_2(x)$

$$\begin{aligned}
 u(x) \rightarrow \varepsilon(x) = u'(x) &= \frac{d}{dx} (u_1 N_1(x) + u_2 N_2(x)) = u_1 N_1'(x) + u_2 N_2'(x) \\
 &= u_1 \left(-\frac{1}{l_e}\right) + u_2 \left(\frac{1}{l_e}\right) = \underbrace{\begin{bmatrix} -\frac{1}{l_e} & \frac{1}{l_e} \end{bmatrix}}_{\underline{\underline{B}}} \underbrace{\begin{bmatrix} u_1 \\ u_2 \end{bmatrix}}_{\underline{\underline{U}}} = \underline{\underline{B}} \underline{\underline{U}}
 \end{aligned}$$

STRAIN-DISPL MATRIX

3) T.P.E.

$$\begin{aligned}
 \Pi^e(u) &= \frac{1}{2} \int_0^{l_e} EA \varepsilon^2 dx - \int_0^{l_e} p u - F_1 u_1 - F_2 u_2 \\
 &= \frac{1}{2} \int_0^{l_e} \underbrace{(\underline{\underline{B}} \underline{\underline{U}})^T}_{\underline{\underline{\varepsilon}}} EA \underbrace{(\underline{\underline{B}} \underline{\underline{U}})}_{\underline{\underline{\varepsilon}}} dx - \int_0^{l_e} p (\underline{\underline{N}} \underline{\underline{U}}) dx - \begin{bmatrix} u_1 & u_2 \end{bmatrix} \begin{bmatrix} F_1 \\ F_2 \end{bmatrix} \\
 & \qquad \qquad \qquad \underline{\underline{F}}
 \end{aligned}$$

$$= \frac{1}{2} \int_0^{l_e} \underline{\underline{U}}^T \underline{\underline{B}}^T EA \underline{\underline{B}} \underline{\underline{U}} dx - \int_0^{l_e} \underline{\underline{U}}^T \underline{\underline{N}}^T p dx - \underline{\underline{U}}^T \underline{\underline{F}}$$

$$= \frac{1}{2} \underline{\underline{U}}^T \underbrace{\left(\int_0^{l_e} \underline{\underline{B}}^T \underline{\underline{B}} EA dx \right)}_{\underline{\underline{K}}^e} \underline{\underline{U}} - \underline{\underline{U}}^T \left(\int_0^{l_e} \underline{\underline{N}}^T p dx \right) - \underline{\underline{U}}^T \underline{\underline{F}}$$

$$= \frac{1}{2} \underline{\underline{U}}^T \underline{\underline{K}}^e \underline{\underline{U}} - \underline{\underline{U}}^T \underbrace{\left[\int_0^{l_e} \underline{\underline{N}}^T p dx - \underline{\underline{F}} \right]}_{\substack{\text{VECTOR OF NORMAL} \\ \text{FORCES } (\underline{\underline{F}}^e)}} = \Pi^e(\underline{\underline{u}}) = \frac{1}{2} \underline{\underline{U}}^T \underline{\underline{K}}^e \underline{\underline{U}} - \underline{\underline{U}}^T \underline{\underline{F}}^e$$

$$x = \frac{x^2}{2le}$$

$$\underline{\underline{K}}^e = EA \int_0^{le} \underline{\underline{B}}^T \underline{\underline{B}} dx = EA \int_0^{le} \begin{bmatrix} -1/le \\ 1/le \end{bmatrix} \begin{bmatrix} -1 & 1 \end{bmatrix} dx$$

$$= \frac{EA}{le^2} \int_0^{le} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} dx = \boxed{\frac{EA}{le} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = \underline{\underline{K}}^e}$$

STIFFNESS MATRIX OF A GENERIC ELEMENT

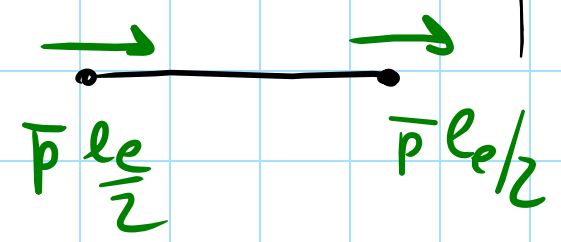
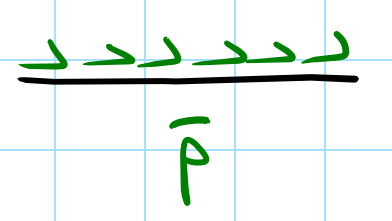
ONLY DISTR. FORCES

$$\underline{\underline{F}}^e = \int_0^{le} p \underline{\underline{N}} dx = \int_0^{le} \underline{\underline{N}}^T p dx = \int_0^{le} \begin{bmatrix} (1-x/le) p(x) \\ x/le p(x) \end{bmatrix} dx$$

IF $p(x)$: CONST \Rightarrow
 $= \bar{p}$

$$= \bar{p} \int_0^{le} \begin{bmatrix} 1-x/le \\ x/le \end{bmatrix} dx = \bar{p} \begin{bmatrix} le/2 \\ le/2 \end{bmatrix}$$

$$\underline{\underline{F}}^e = \begin{bmatrix} \bar{p} le/2 \\ \bar{p} le/2 \end{bmatrix}$$



(WITH LINEAR SHAPE FUNCTION)

The general vector

$$\underline{\underline{F}}^e = \begin{bmatrix} \int_0^{le} (1-x/le) p(x) dx + F_1 \\ \int_0^{le} x/le p(x) dx + F_2 \end{bmatrix}$$

Up to now $\Pi^e(\underline{U}) = \frac{1}{2} \underline{U}^T \underline{\tilde{K}}^e \underline{U} - \underline{U}^T \underline{F}^e$

P.E. STATIONARY $\frac{\partial \Pi^e}{\partial \underline{U}} = \underline{0} \implies \underline{\tilde{K}}^e \underline{U} - \underline{F}^e = \underline{0} \implies \boxed{\underline{\tilde{K}}^e \underline{U} = \underline{F}^e}$