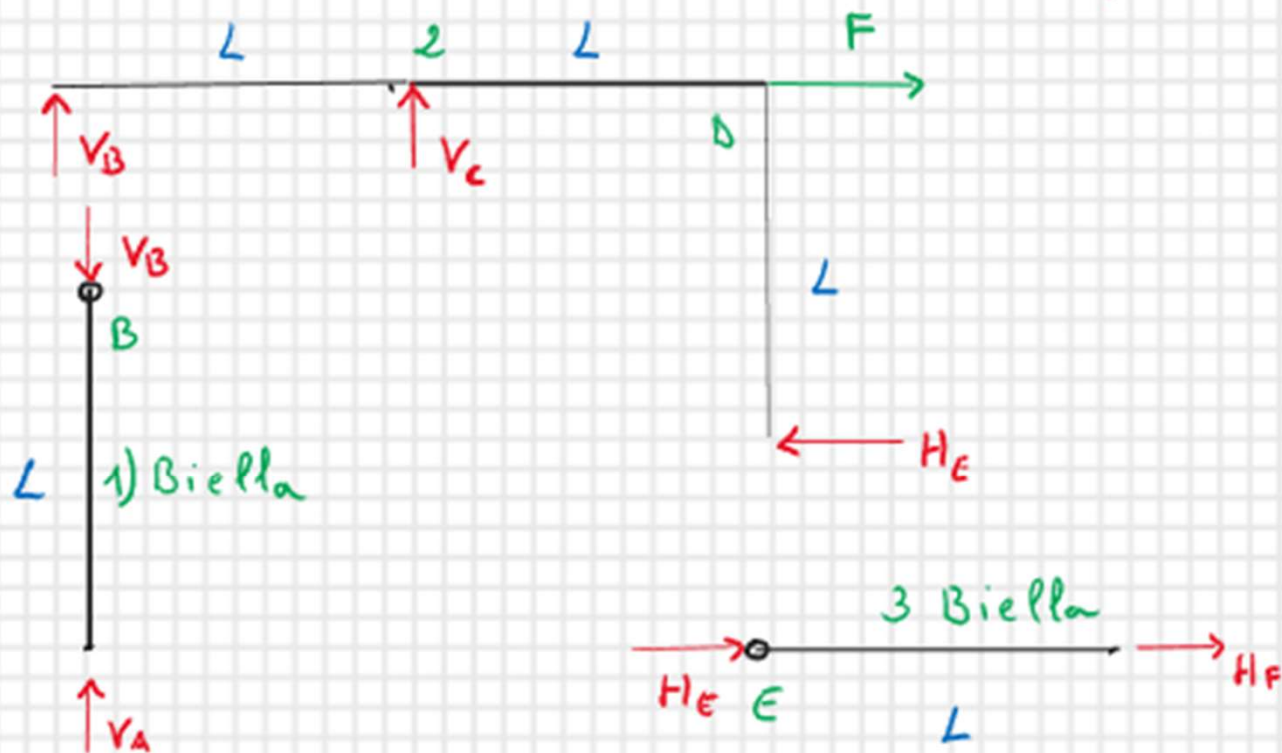
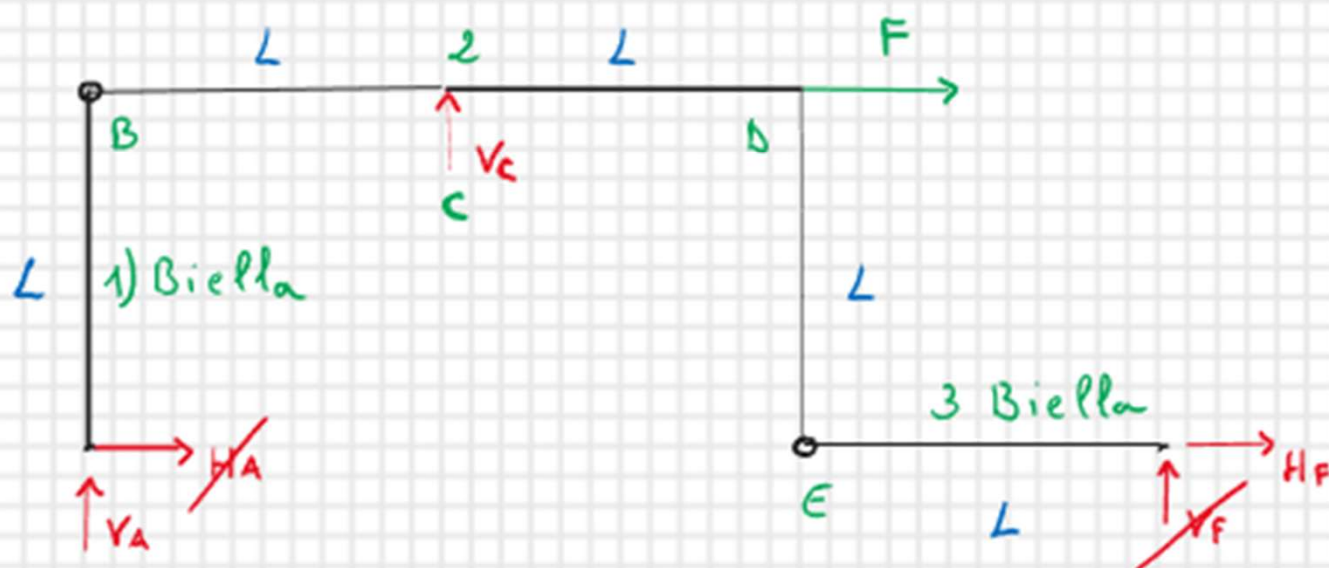
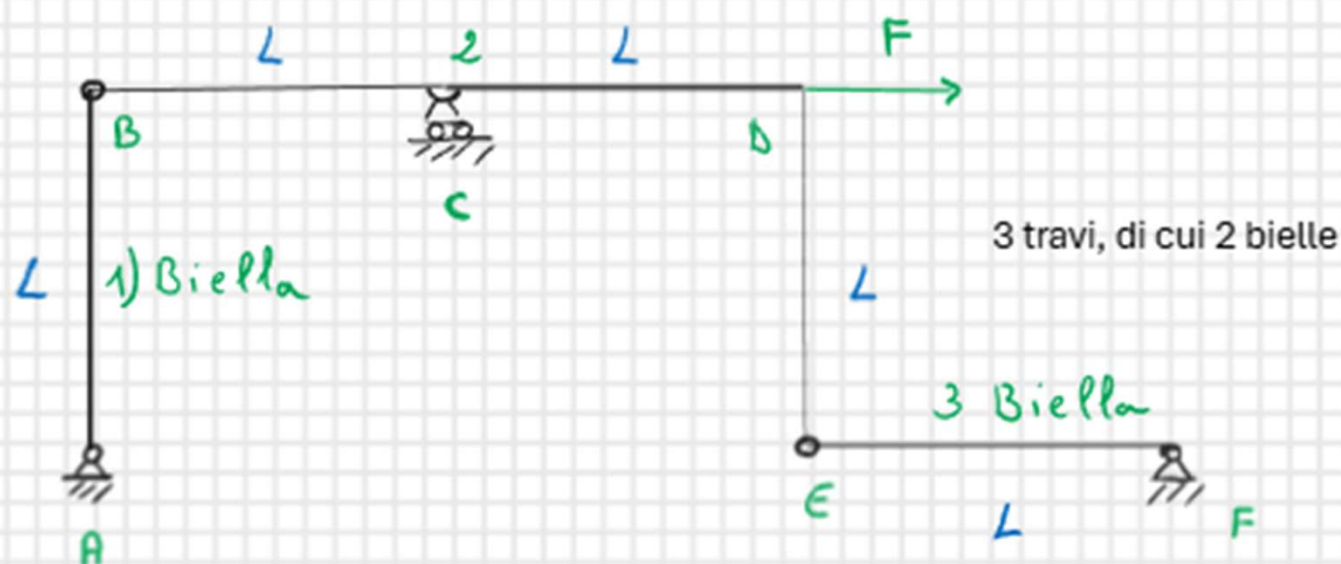
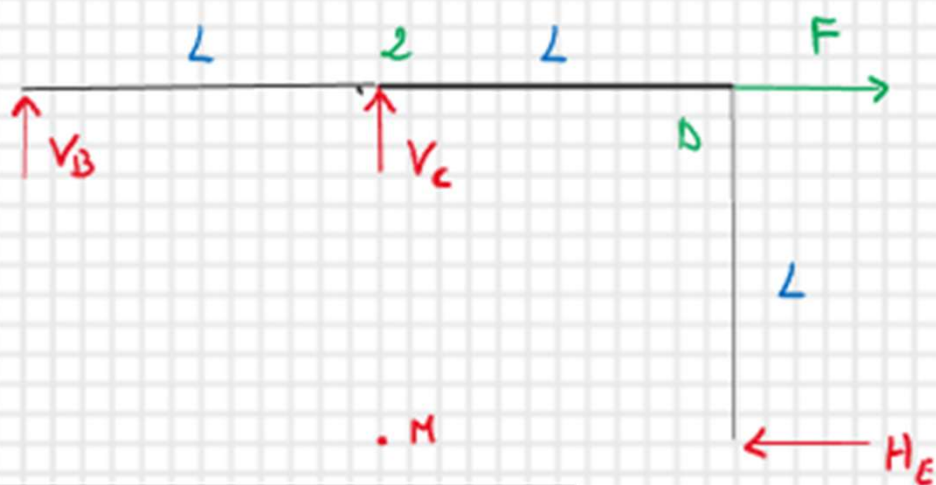


Esempio di struttura con biella



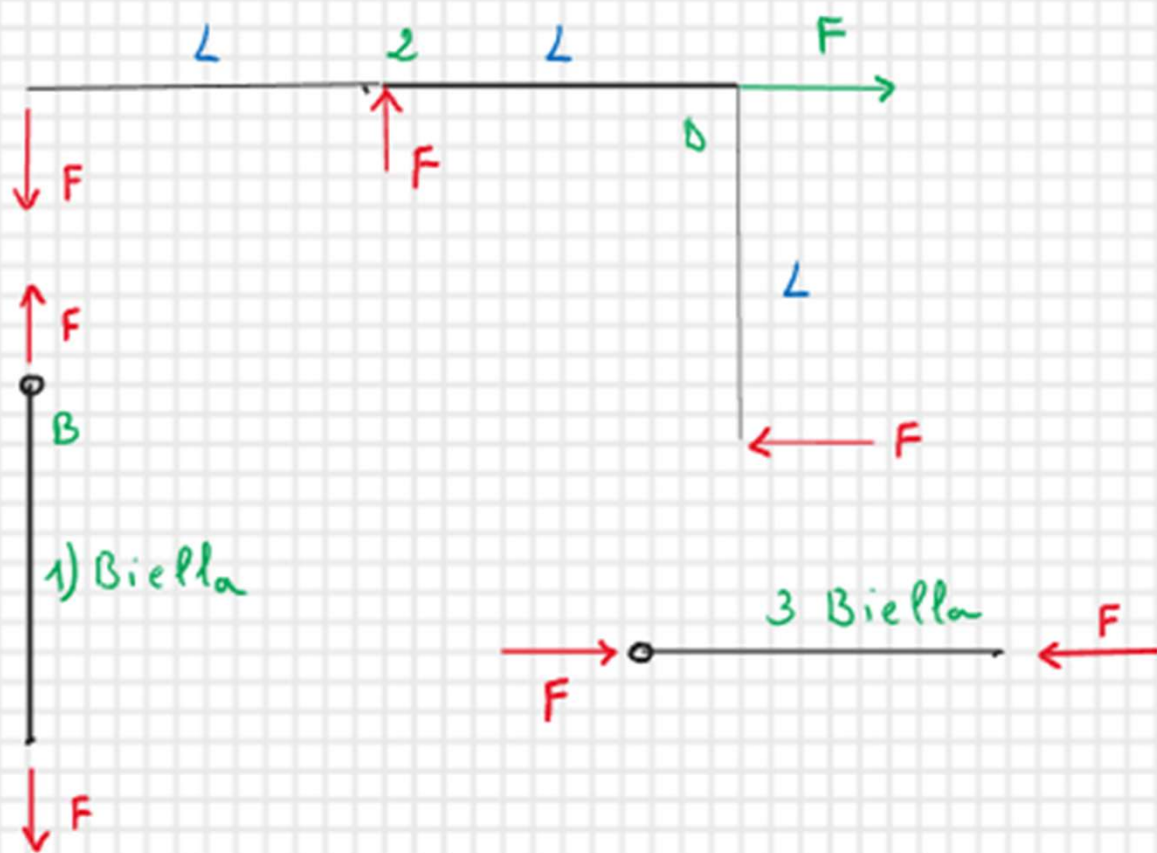


Risolvo la trave 2

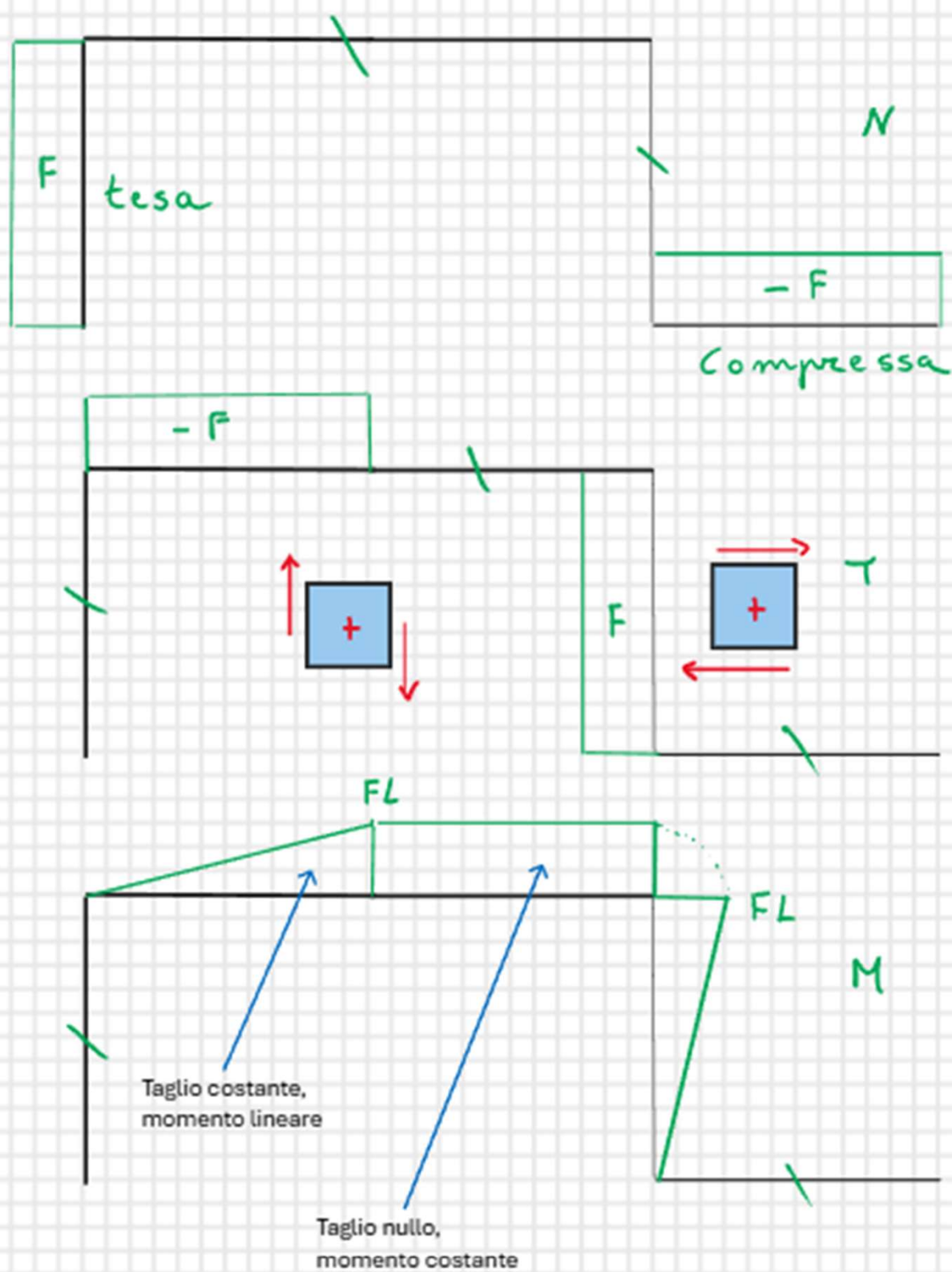
$$H_E = F$$

$$V_B + V_C = 0$$

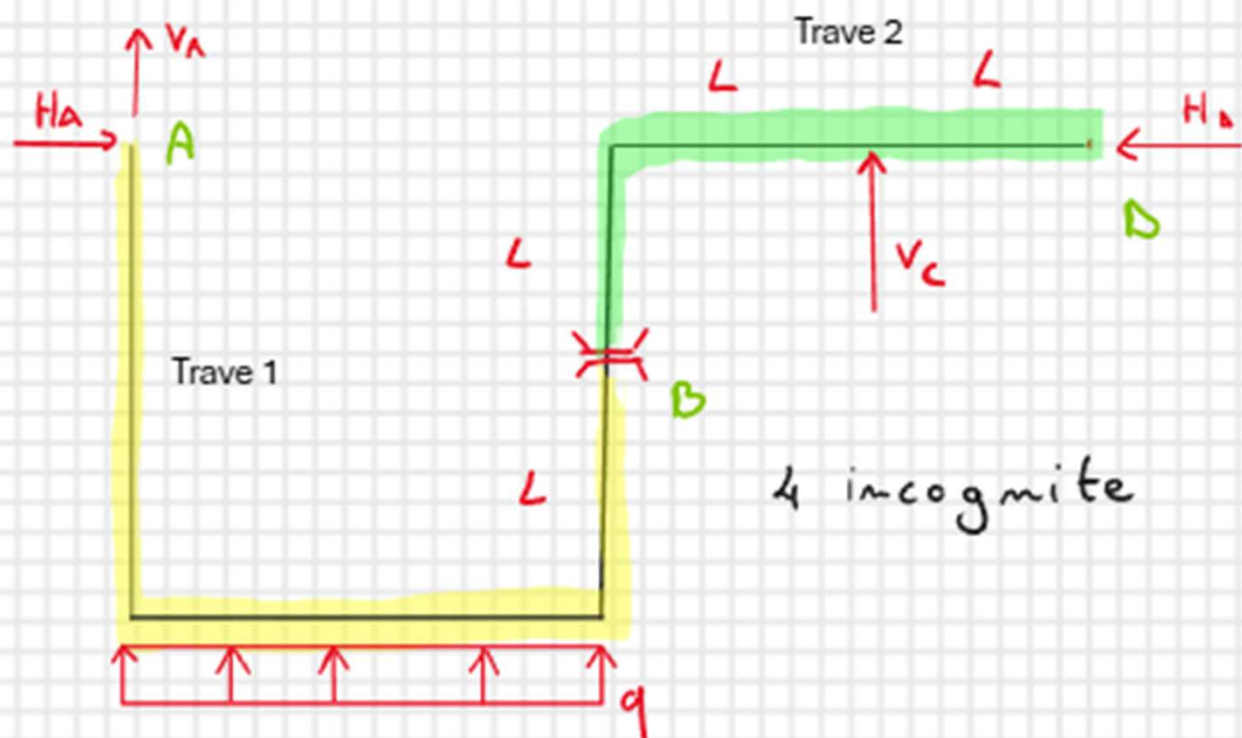
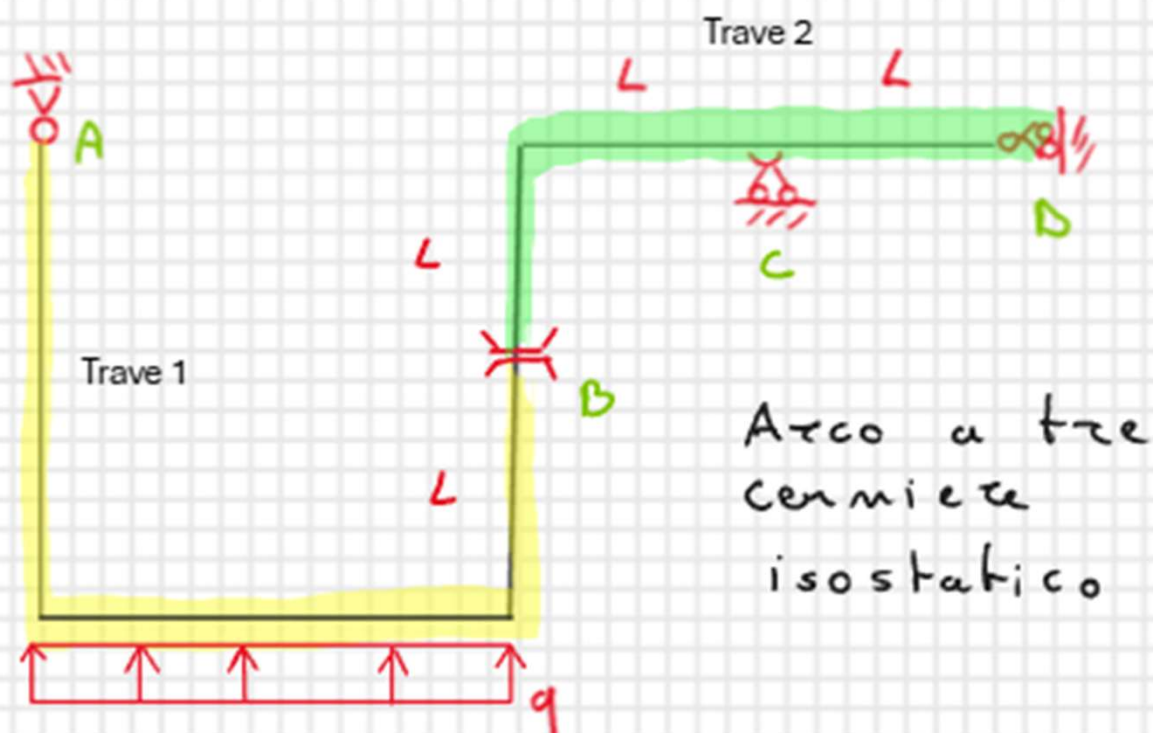
$$V_B L + FL = 0 \quad \text{in } M$$



Diagrammi



Esempio



Eq. c. statica

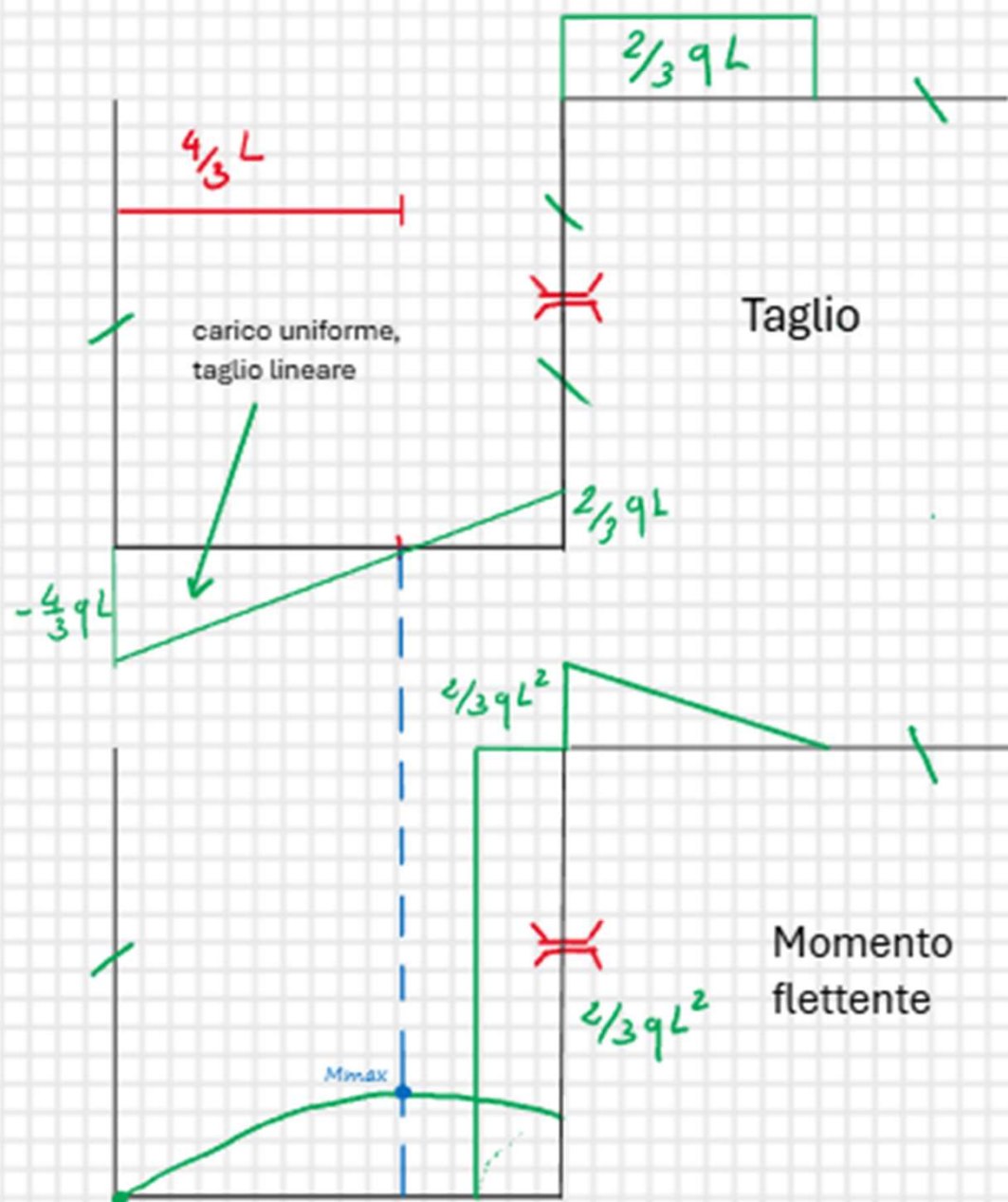
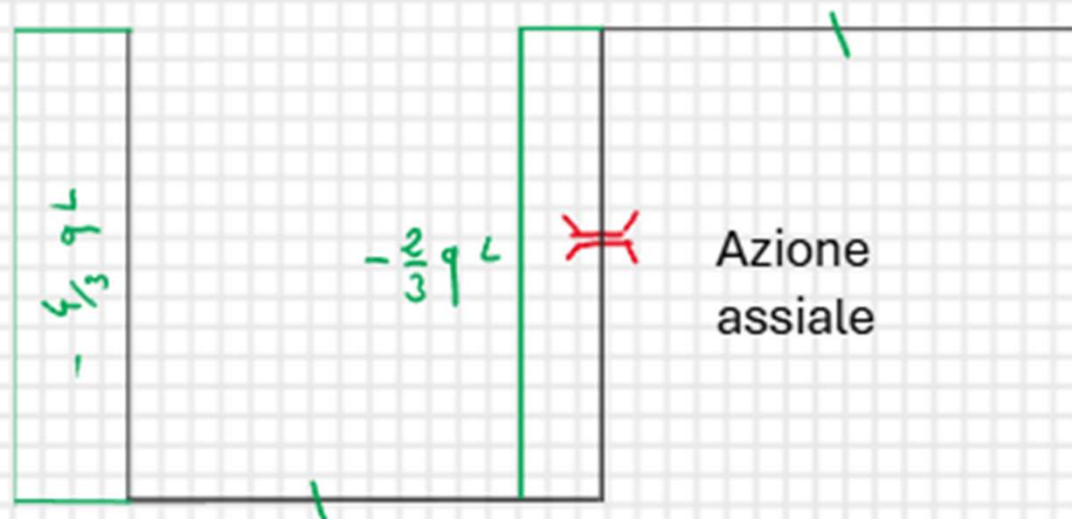
$$H_A - H_D = 0 \quad 1)$$

$$V_A + V_C + 2qL = 0 \quad 2)$$

$$\text{in A} \quad 2qL^2 + 3V_C L = 0 \quad 3)$$

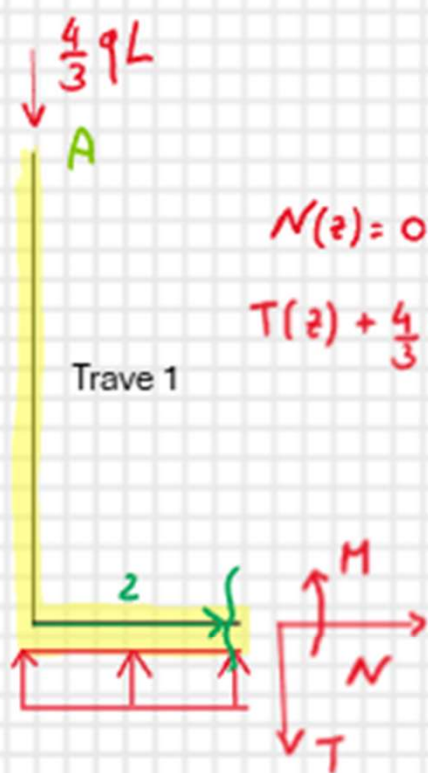
Aggiungo un'equazione ausiliaria

Diagrammi



Carico uniforme, taglio lineare, momento parabolico

Potete sempre scrivere le equazioni di equilibrio relative al tratto di trave in esame



$$N(z) = 0$$

$$T(z) + \frac{4}{3}qL - qz = 0 \longrightarrow T(z) = -\frac{4}{3}qL + qz$$

$$T(z=0) = -\frac{4}{3}qL$$

$$T(z=2L) = \frac{2}{3}qL$$

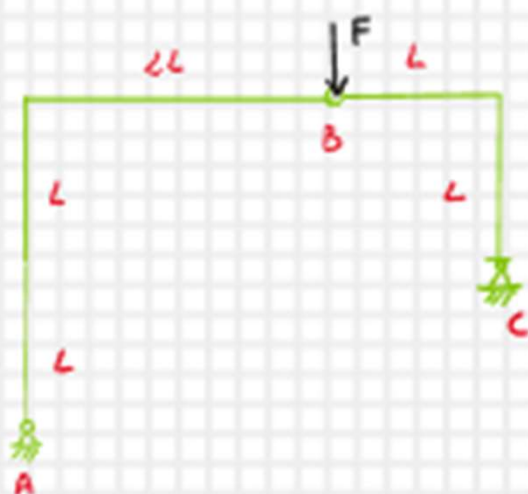
Taglio nullo

$$-\frac{4}{3}qL + qz = 0 \longrightarrow z = \frac{4}{3}L$$

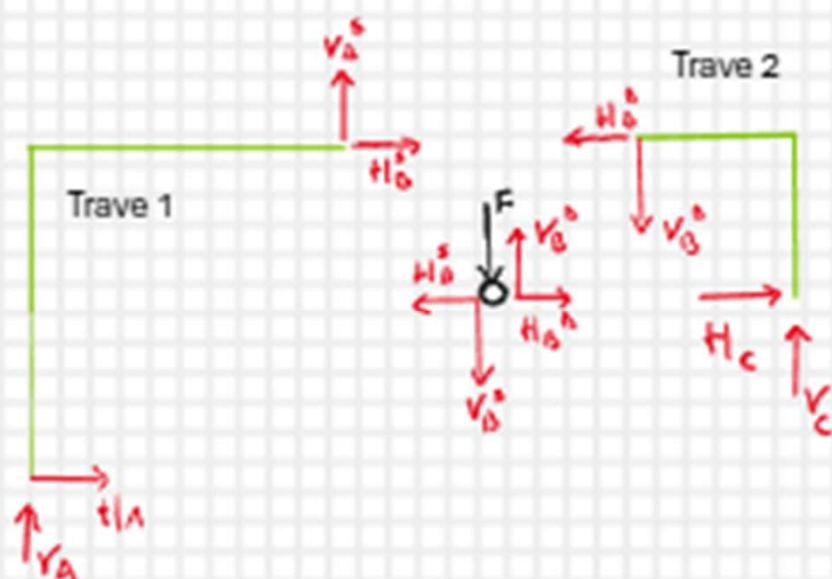
$$M(z) + \frac{4}{3}qLz - q\frac{z^2}{2} = 0 \longrightarrow M(z) = -\frac{4}{3}qLz + q\frac{z^2}{2}$$

Carico applicato sulla cerniera

Isolo il nodo B nel quale è applicata la forza concentrata



S e D per indicare la sezione a sinistra e a destra del nodo



Abbiamo 8 incognite ($H_A, V_A, H_B^s, V_B^s, H_B^d, V_B^d, H_C, V_C$)

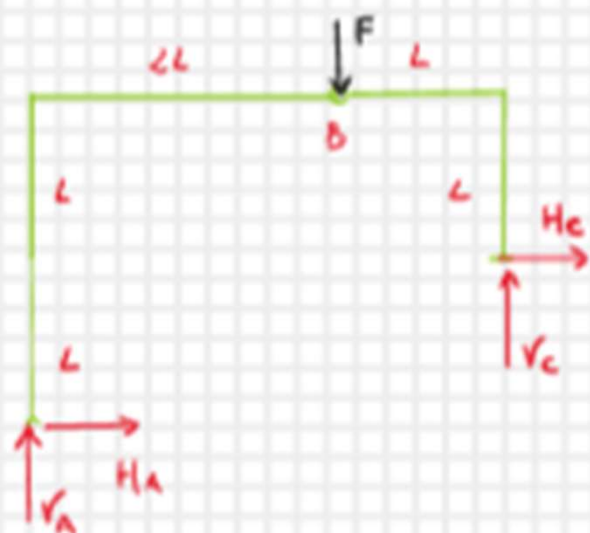
3 eq. cardinali per ciascuna trave = 6

Dobbiamo aggiungere 2 equazioni:

Equazioni di equilibrio alla traslazione del nodo

Eq. orizzontale 1) $H_B^d - H_B^s = 0$

Eq. verticale 2) $V_B^s + F - V_B^d = 0$

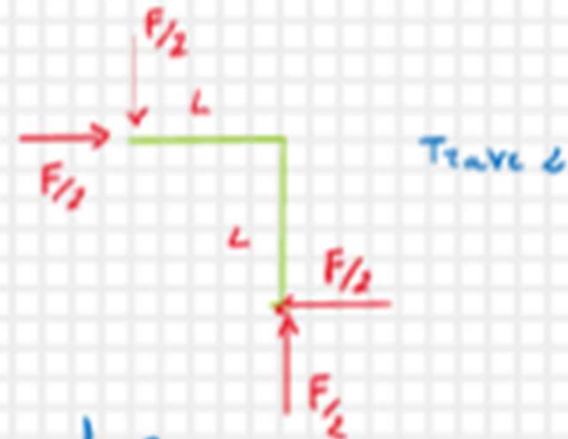
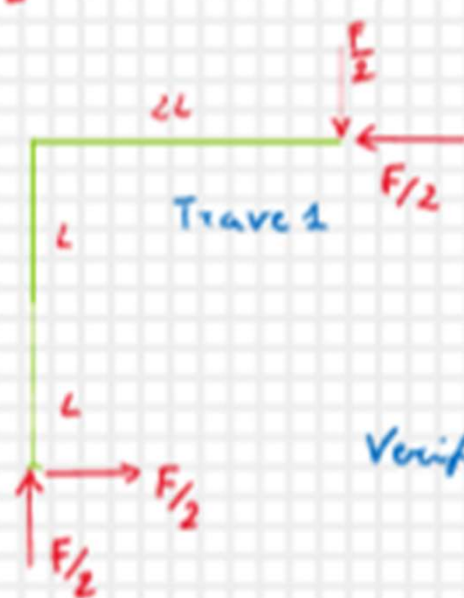
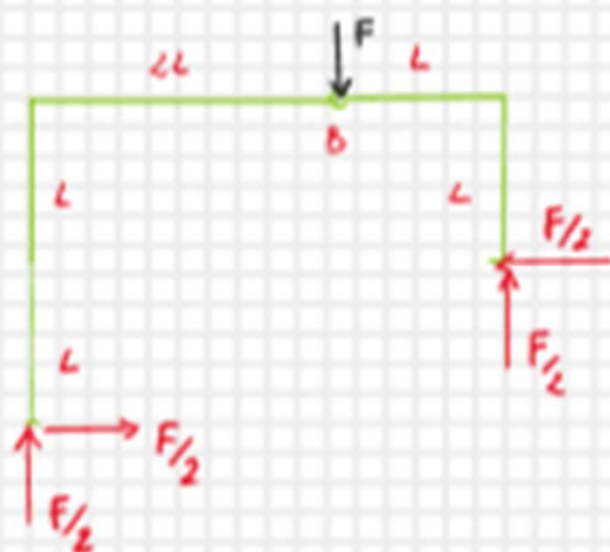


$$1) H_A + H_C = 0$$

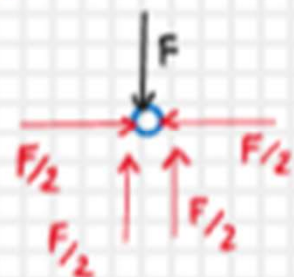
$$2) V_A + V_C = F$$

$$3) \text{In } A \quad V_C 3L - 2FL - H_C L = 0$$

$$4) V_C L + H_C L = 0 \quad \text{Auxiliaria}$$



Verifica nel nodo B



Eq. equilibrio nodo

$$1) F/2 - F/2 = 0 \quad \circ$$

$$2) F - F/2 - F/2 = 0 \quad \vee$$

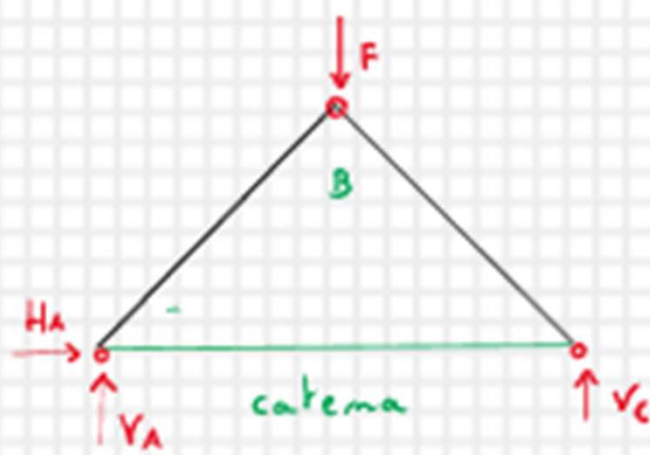
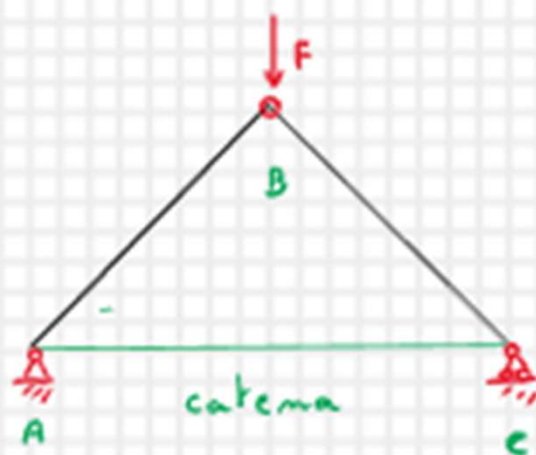
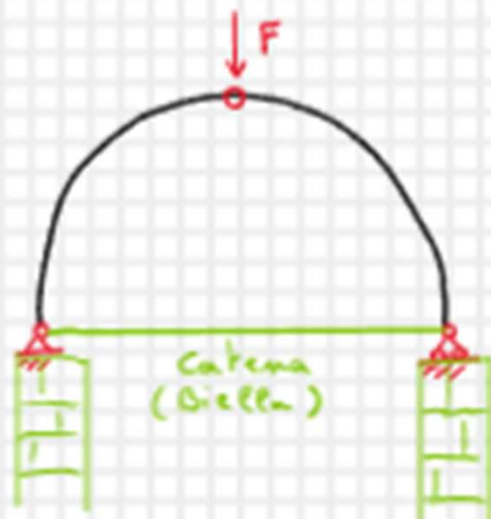
Eq. del nodo verificato

Esempio per arrivare alle strutture chiuse



Modifico la cerniera con un carrello, introduco una biella per garantire l'astaticità. (Genero una struttura chiusa)

modifico

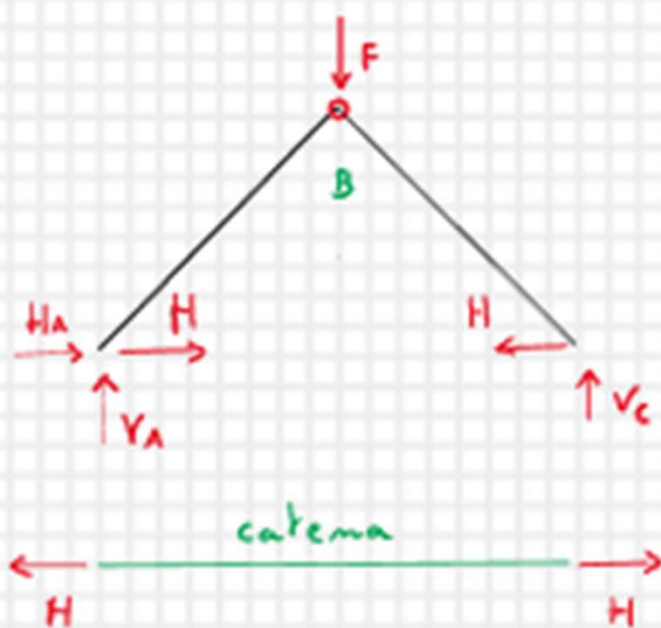


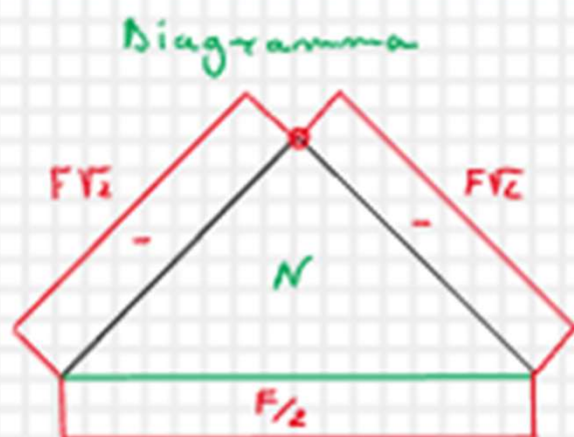
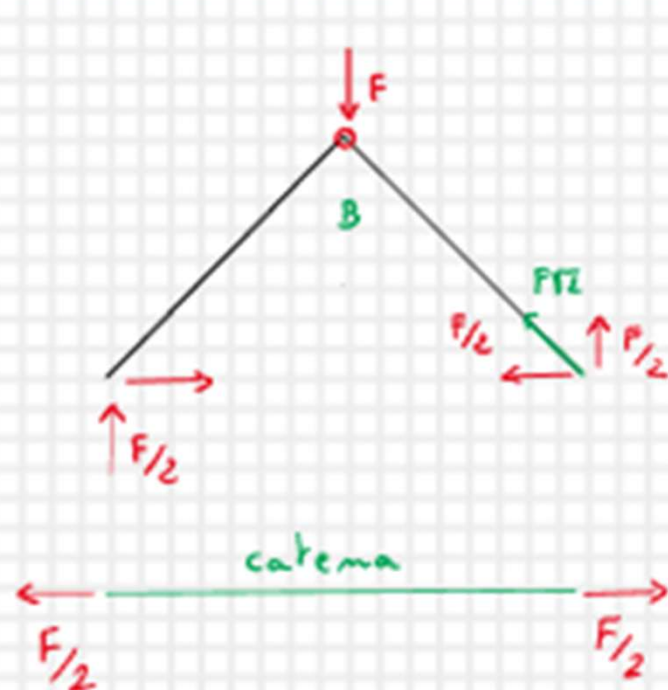
Equazioni di equilibrio

- 1) $H_A = 0$ $H_A = 0$
- 2) $V_A + V_C = F$ $V_A = F/2$
- 3) $FL - V_C L = 0$ $V_C = F/2$

Per ricavare la forza H nella biella utilizzo un'equazione ausiliaria

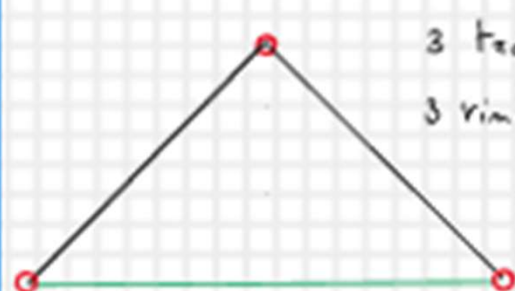
$$4) V_C L - H L = 0$$





3 Bielle, taglio e momento flettente nullo

Triangolo isostatico



3 cerni

3 vincoli relativi doppi

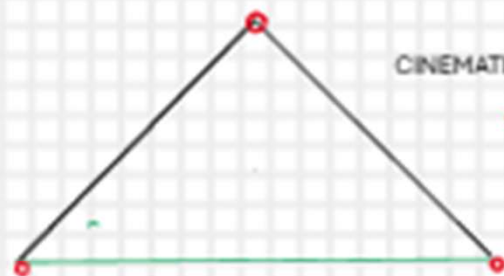
Le tre cerniere reali o "fittizie" non devono essere allineate



Attenzione, non è un triangolo isostatico

$$gl = 3 \cdot 3 = 9 \quad m_{tot} = 2 \cdot 3 = 6$$

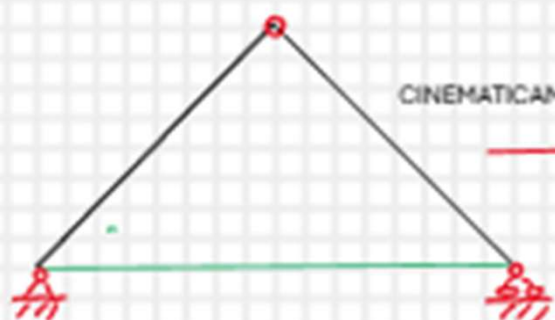
$gl_{residua} = 9 - 6 = 3 \longrightarrow$ Si comporta come un corpo rigido



CINEMATICAMENTE EQUIVALENTI



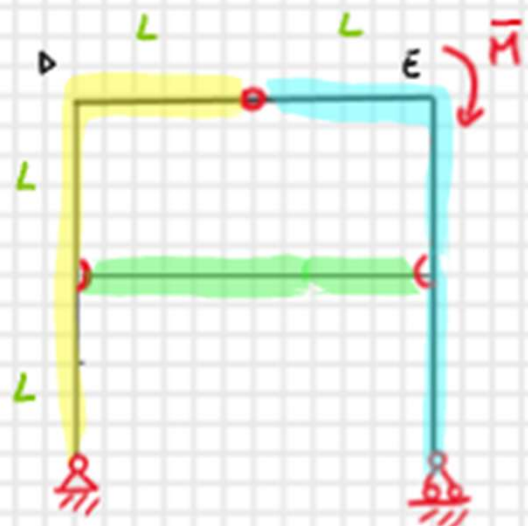
corpo rigido



CINEMATICAMENTE EQUIVALENTI



Strutture chiuse

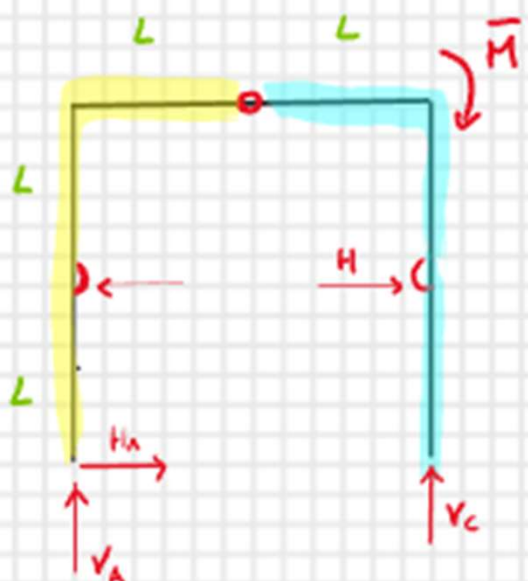


Riconosciamo un triangolo isostatico: 3 travi con 3 vincoli doppi interni

CINEMATICAMENTE EQUIVALENTI



isostatica



Eq. di equilibrio

$$1) H_A = 0$$

$$H_A = 0$$

$$2) V_A + V_C = 0$$

$$V_A = -\frac{\bar{M}}{2L}$$

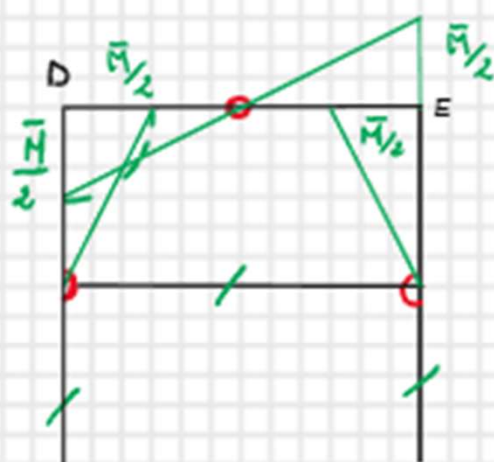
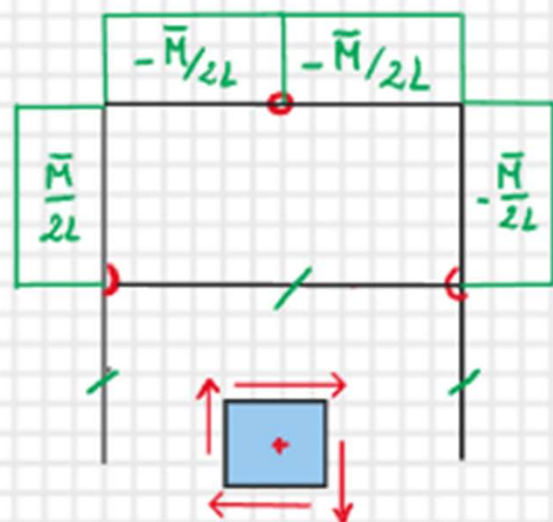
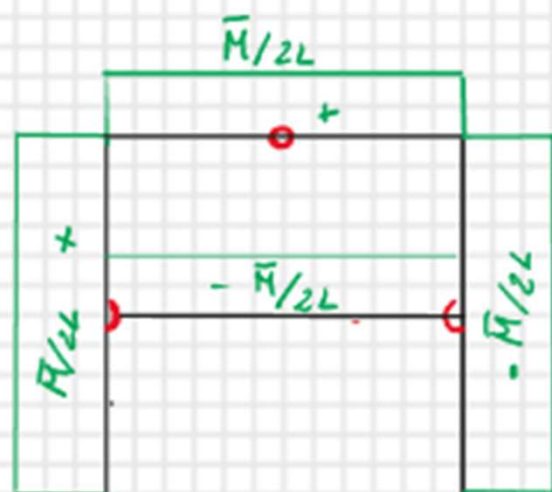
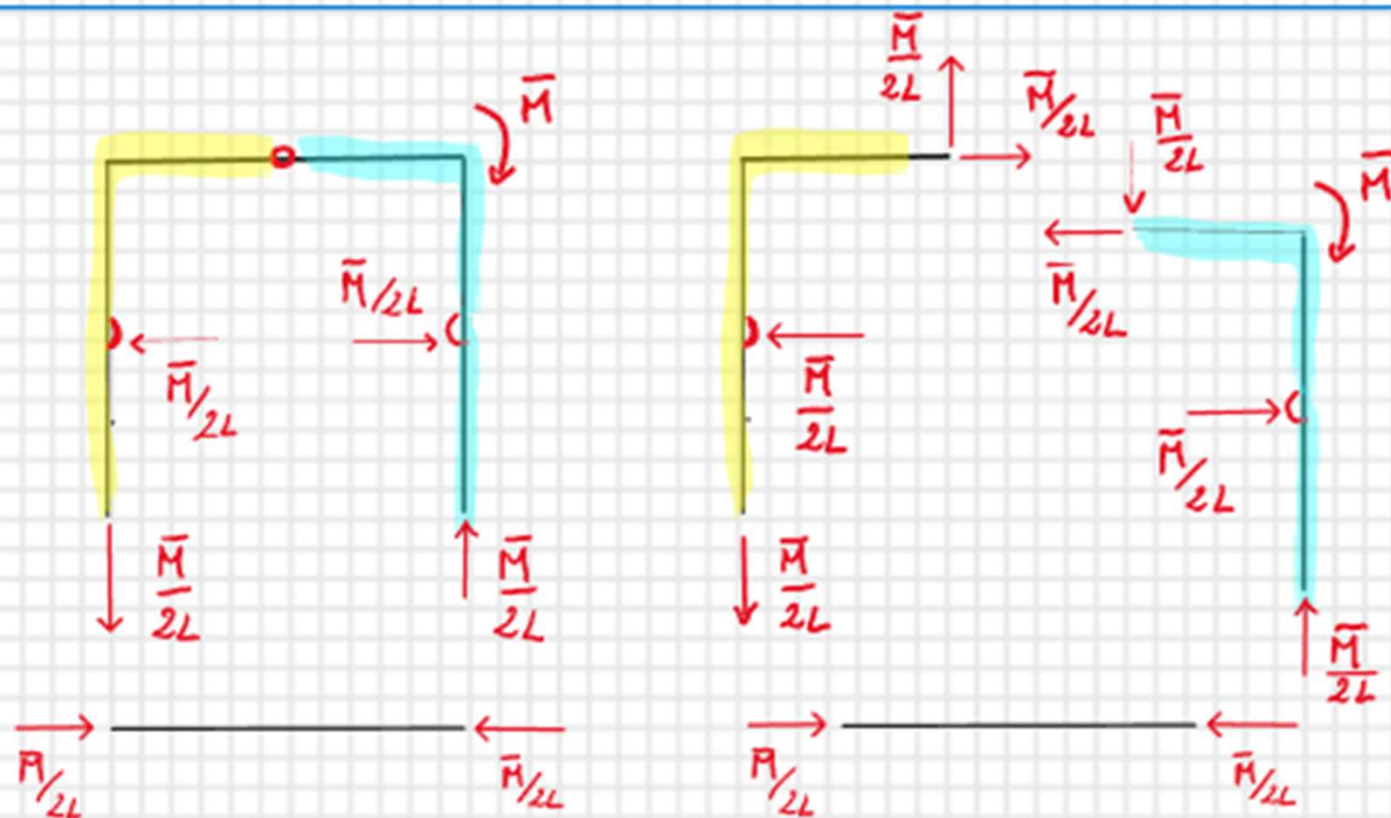
$$3) \text{ in A } \bar{M} - 2V_C L = 0$$

$$V_C = \frac{\bar{M}}{2L}$$

Calcolare H (eq. trave 1)

$$4) HL + V_A L - 2H L = 0$$

$$H = \frac{\bar{M}}{2L}$$



Momento lineari,
taglio costante

Verifica dei nodi

Nodo b

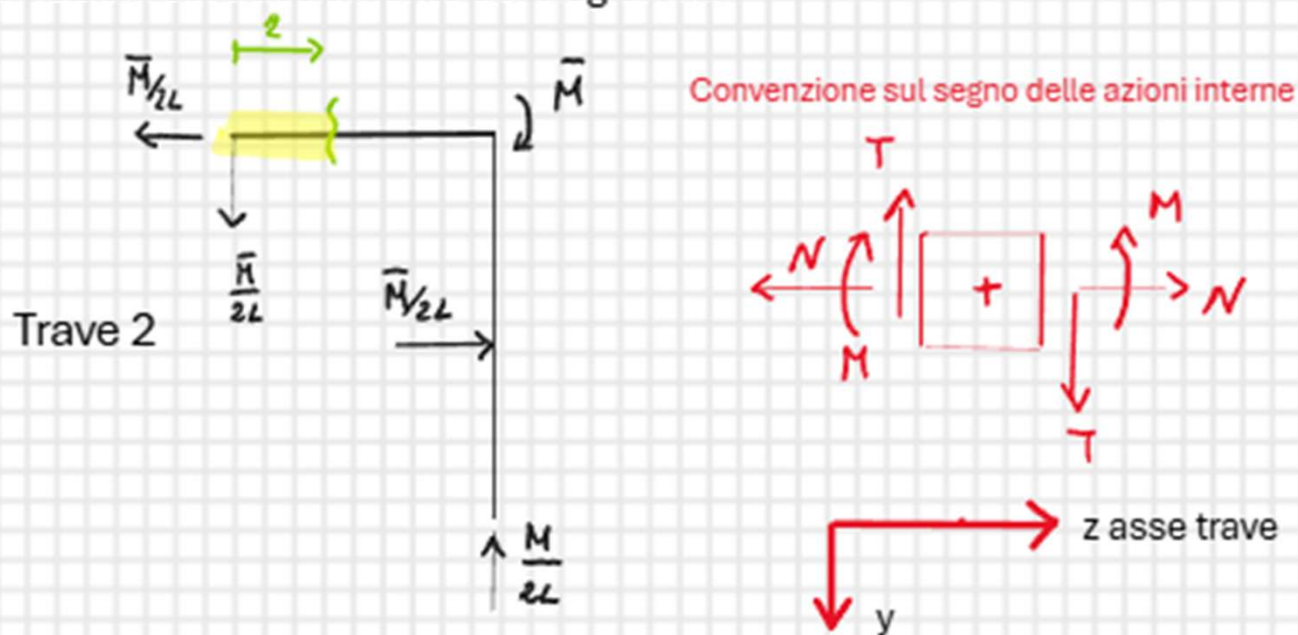


Nodo E



Eq. momenti soddisfatto

Osservazioni sul tracciamento dei diagrammi



Convenzione sul segno delle azioni interne

- Nelle lezioni precedenti imponevamo l'equilibrio della parte di trave individuata dalla sezione

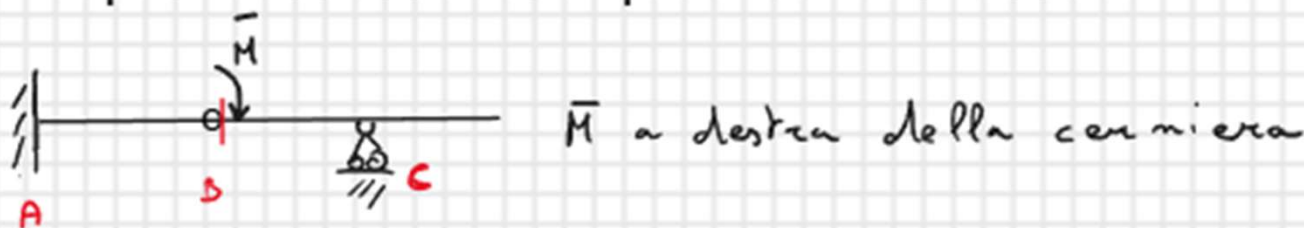
A free body diagram of a beam segment of length z is shown on the left, with internal forces N (normal), T (shear), and M (moment) acting on its right face. To the right, the equilibrium equations are listed:

$$\begin{aligned} -\frac{\bar{H}}{2L} + N(z) &= 0 & N(z) &= \frac{\bar{H}}{2L} \\ -\frac{\bar{M}}{2L} - T(z) &= 0 & \longrightarrow T(z) &= -\frac{\bar{M}}{2L} \\ -M(z) - \frac{\bar{H}}{2L} z &= 0 & M(z) &= -\frac{\bar{H}}{2L} z \end{aligned}$$

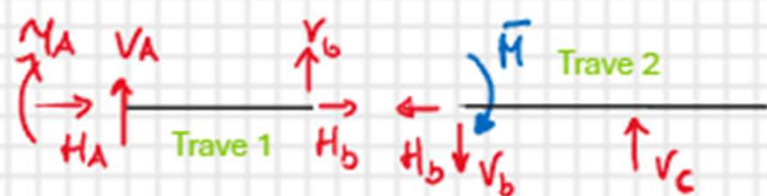
Le azioni interne utilizzate sono quelle della faccia del concio elementare opposta al tratto di trave individuato dalla sezione (faccia rossa)

Il momento negativo delle formule lo disegniamo dal lato opposto alla freccia delle y rivolta in basso nella convenzione (o sul lato delle fibre tese)

Esempi di strutture oltre a quelli dei temi d'esame



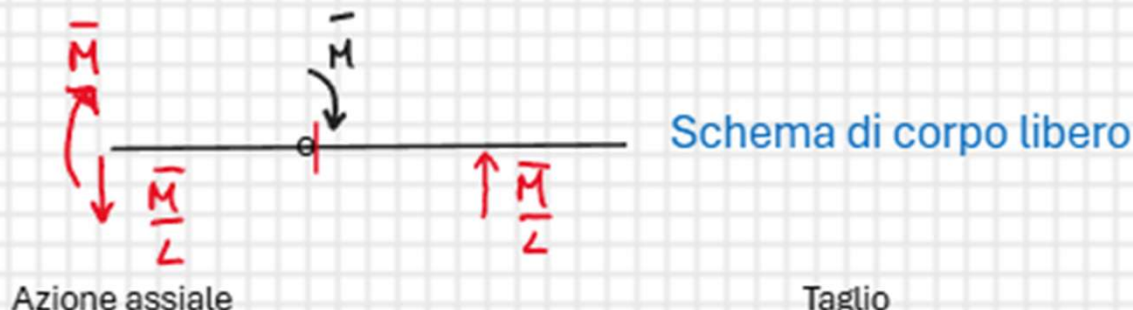
Risolve prima la trave 1 (3 incognite)



$$\begin{aligned} H_b &= 0 & H_b &= 0 \\ V_b &= V_c & V_c &= \bar{M}/L \\ -\bar{M} + V_c L &= 0 & V_b &= \bar{M}/L \end{aligned}$$

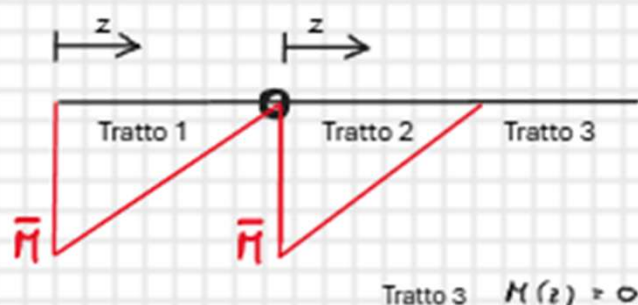
Risolve la struttura 2, calcolo b 1

$$\begin{aligned} H_A &= 0 & V_A &= -\bar{M}/L & M_A &= \frac{\bar{M}}{L} L = \bar{M} \end{aligned}$$



Taglio

Momento flettente



$$\begin{aligned} \bar{M}(z) - \bar{M} + \frac{\bar{M}}{L}(L+z) &= 0 \\ \bar{M}(z) &= \bar{M} - \frac{\bar{M}}{L} z \end{aligned}$$

Alternativa tratto 2 con la z che parte da destra

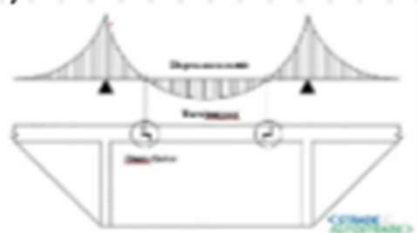
$$\bar{M}(z) = \frac{\bar{M}}{L} z$$

$$\begin{aligned} \bar{M}(z) - \bar{M} + \frac{\bar{M}}{L} z &= 0 \\ \bar{M}(z) &= \bar{M} - \frac{\bar{M}}{L} z \end{aligned}$$

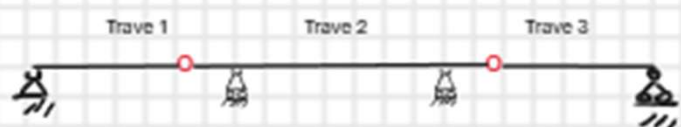
$$\text{Tratto 3 } \bar{M}(z) = 0$$

Travi Gerber (usate negli impalcati dei ponti)

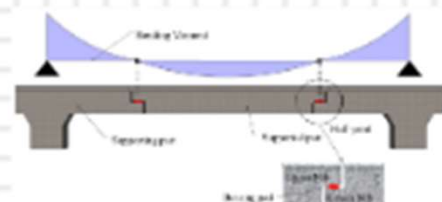
schema n° 1



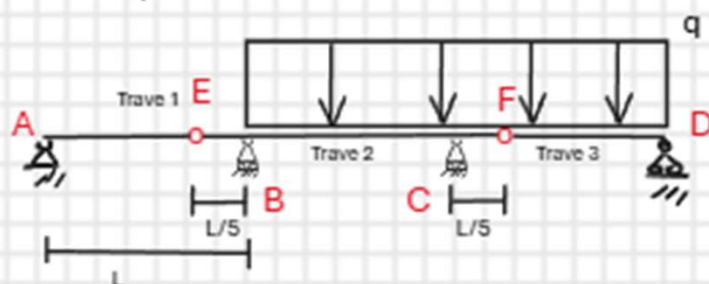
schema n° 2



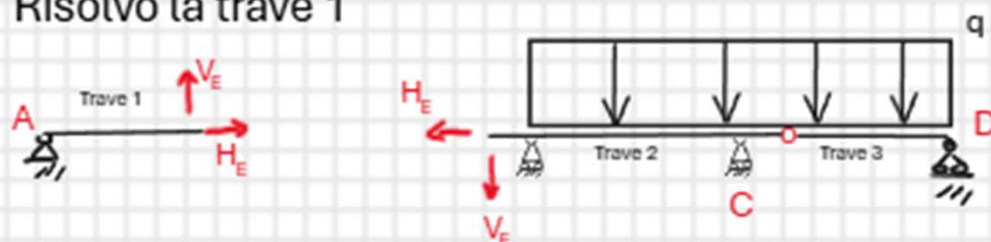
Isostatiche



Esempio



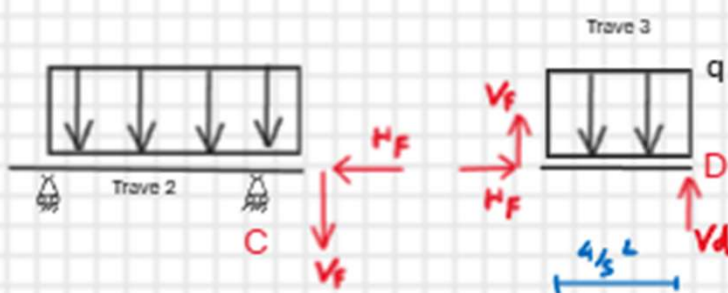
Risolvero la trave 1



Trave 1 scarica

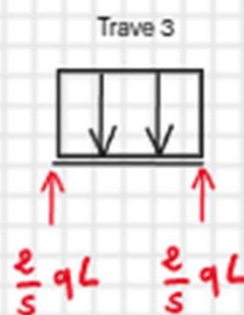
$$H_E = 0 \quad V_E = 0$$

Risolvero la trave 3

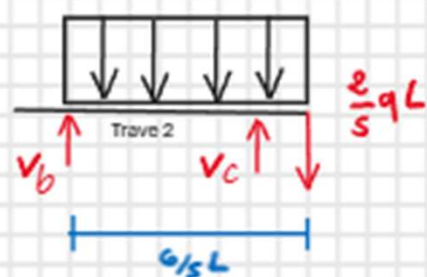


$$H_F = 0$$

$$V_F = V_D = \frac{2}{5} qL$$

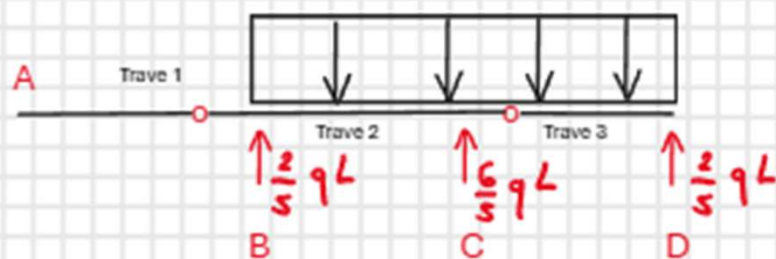


Risolvero la trave 2



$$V_b = \frac{2}{5}qL \quad V_c = \frac{6}{5}qL$$

Schema di corpo libero



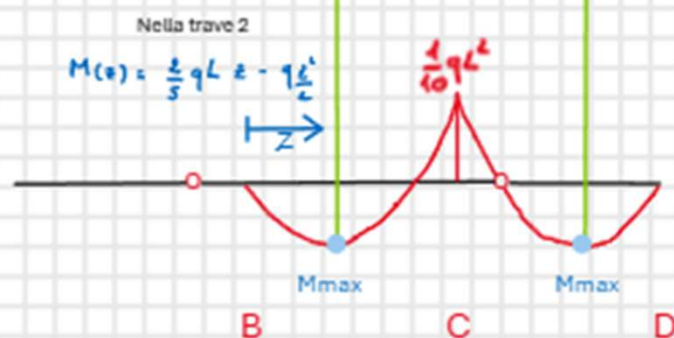
Azione assiale

$$N(z) = 0$$

Taglio



Momento flettente



Nella trave 2

$$M(z) = \frac{2}{5}qLz - \frac{1}{2}qz^2$$