

ADVANCED STRUCTURAL MECHANICS 2 (3 ECTS)
CFU) ASM2 24/3/26

STARTING FROM THE MODULE: MECCANICA AVANZATA E DINAMICA DELLE
STRUTTURE (1° SEMESTER) (MADS)

MADS: - ELASTIC LINE (IV ORDER EQUATION / E-BERNOULLI)

- WORK-ENERGY THEOREMS (CAPEYRON, BETTI, CASTIGLIANO)

- LIMIT ANALYSIS (NOTION OF PLASTICITY / COLLAPSE OF STRUCTURES)

- BUCKLING INSTABILITY (IV ORDER EQ / GENERAL B. CONDITIONS)

- INTRO TO STRUCTURAL DYNAMICS (1 DOF + SPECTRA)

ASM2: - DYNAMICS OF SYSTEM OF N DOFS → MODAL ANALYSIS

- SOME ISSUES RELATED TO STIFFNESS MATRIX IN CIVIL STRUCT.

- SHEAR-WALL INTERACTION WITH A FRAME MECHANICS

- MASONRY MODELLING / NO TENSION MATERIAL

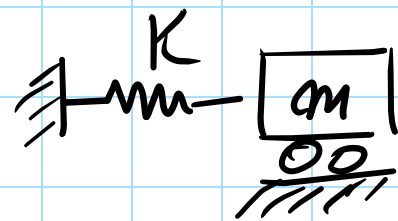
— DOMES, TANKS

— DESIGN PROBLEMS IN WIND TURBINE STRUCT.

— TUNED-MASS DAMPERS / BRIEF INTRO TO "METAMATERIALS"

LET US START WITH DYNAMICS OF MULTI-D.O.F. SYSTEMS

WHAT WE HAVE LEARNED FROM THE AUTUMN MODULE:



FREE OSCILLATIONS OF A RESONATOR

$$\omega = \sqrt{\frac{K}{m}}$$

$$f = \frac{\omega}{2\pi}$$

$$T = \frac{2\pi}{\omega} = \frac{1}{f}$$

CIRCULAR FREQUENCY (rad/s)
(PULSAZIONE)

FREQUENCY (Hz)
(FREQUENZA)

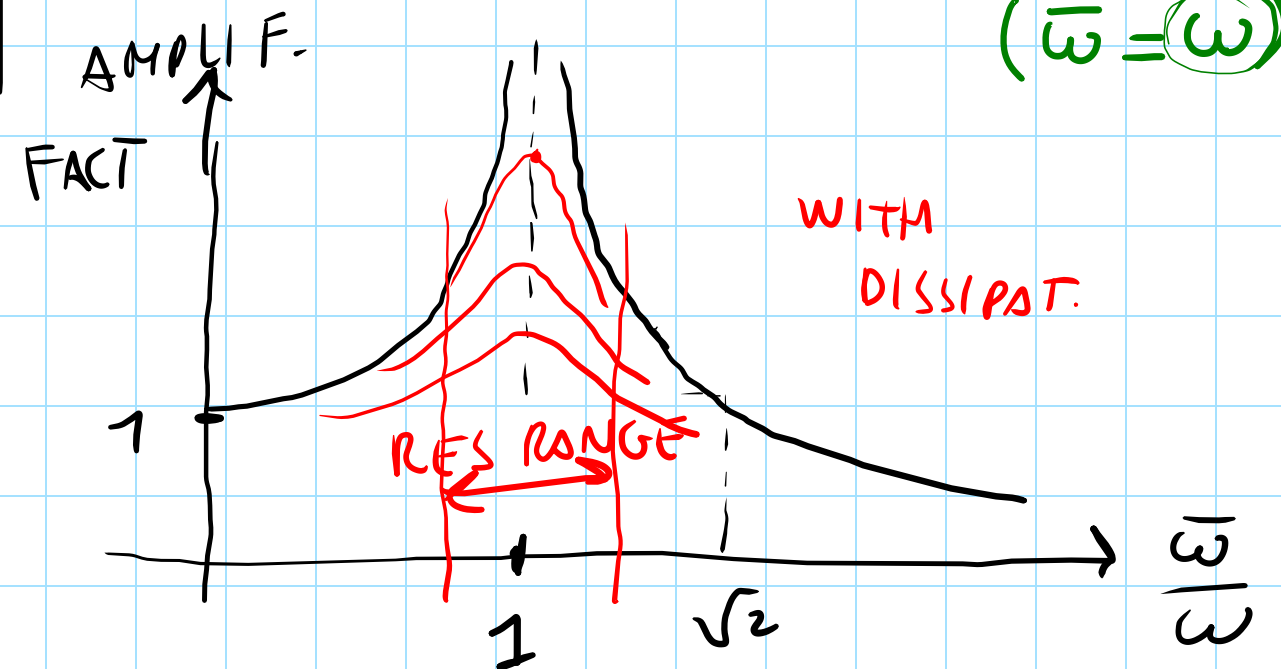
PERIOD (s)



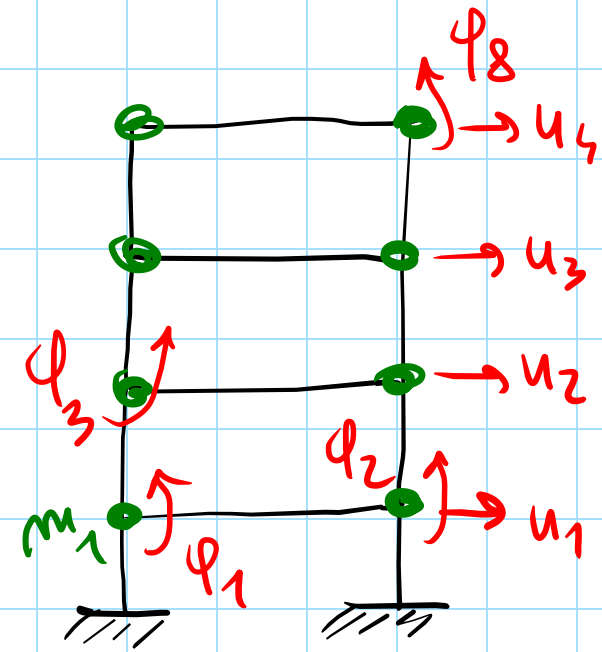
FORCED VIBRATIONS



RISK OF RESONANCE ($\bar{\omega} = \omega$)

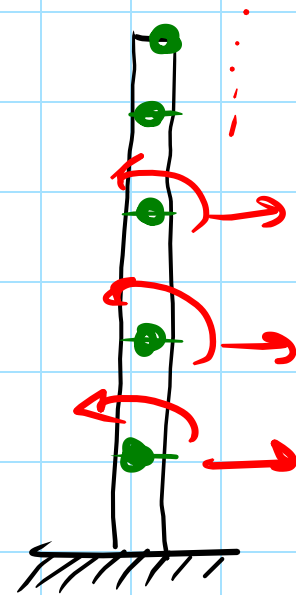


TYPICAL M-DOF STRUCTURES IN CIVIL ENGINEERING



FRAME

(DISCR WITH
12 DOFS
(4 \rightarrow DISPL.
8 \rightarrow ROTATIONS)



TOWER

MAINLY TWO APPROACHES: - CONTINUOUS SYSTEMS
(BEAMS AND BARS)

- MDOFS WITH SELECTED
D.O.Fs (DISPL. / ROTAT.)

IN THE NEXT SLIDES WE STUDY FREE OSCILLATIONS
OF A GENERIC MDOF SYSTEM (NO DAMPING)

FREE OSCILLATIONS OF A MDOF-SYSTEM (NO DAMPING) (N : n° OF DOFS)

$$\begin{cases} \underline{\tilde{M}} \underline{\ddot{u}} + \underline{\tilde{K}} \underline{u} = \underline{0} \quad (1) & \underline{u}(t): \text{vector of } N \text{ DOFS} ; \underline{\tilde{M}}: N \times N \text{ matrix of MASSES} \\ \underline{u}(0) = \underline{u}_0 & \\ \underline{\dot{u}}(0) = \underline{\dot{u}}_0 & \end{cases} \quad \underline{\tilde{K}}: N \times N \text{ STIFFNESS MATRIX}$$

} $2N$ DATA (INITIAL CONDITIONS)
(NOTE THAT THE CONSTANTS TO BE DETERMINED ARE $n^\circ = 2N$)

WE SEEK SOLUTIONS IN THE FORM

$$\underline{u}(t) = \underline{\phi} (A \cos \omega t + B \sin \omega t) \quad ; \quad \underline{\ddot{u}}(t) = -\omega^2 \underline{\phi} (A \cos \omega t + B \sin \omega t)$$

$\underline{\phi}$ VECTOR OF AMPLITUDES *

IN EQ. (1) $-\omega^2 \underline{\tilde{M}} \underline{\phi} (*) + \underline{\tilde{K}} \underline{\phi} (*) = \underline{0} \quad (\forall t) \Rightarrow \boxed{(-\omega^2 \underline{\tilde{M}} + \underline{\tilde{K}}) \underline{\phi} = \underline{0}}$

CLASSICAL EIG. PROBL.

$$(\underline{A} - \lambda \underline{I}) \underline{\phi} = \underline{0}$$

ω^2 ARE "EIGENVALUES"

$\underline{\phi}$ ARE "EIGENVECTORS" ← GENERALISED EIGENVALUE PROBLEM

HOMOGENEOUS SYSTEM; NON TRIVIAL SOLUTION ONLY IF

$$\det(-\omega^2 \underline{\tilde{M}} + \underline{\tilde{K}}) = 0$$

CHARACT. EQ: $d_N (\omega^2)^N + d_{N-1} (\omega^2)^{N-1} + \dots + d_0 = 0$

\Rightarrow N SOLUTIONS: $(\omega^2)_1, (\omega^2)_2, \dots, (\omega^2)_N$

WE USUALLY PUT IN THE ORDER SUCH THAT $0 \leq \omega_1 \leq \omega_2 \leq \omega_3 \leq \dots \leq \omega_N$
(LIST OF CIRCULAR FREQUENCY OF THE SYSTEM)

ω_1 \rightarrow FUNDAMENTAL OR NATURAL CIRCULAR FREQUENCY

$\omega_i \rightarrow T_i = \frac{2\pi}{\omega_i} \rightarrow T_1 \geq T_2 \geq T_3 \dots \geq T_N > 0$

NATURAL / FUNDAMENTAL PERIOD

$\omega_i \rightarrow \underline{\phi}_i$ (EIGENVECTOR): i -th VIBRATION MODE
(i -SIMO MODE OF VIBRATE)

USUALLY $\underline{\phi}_i$ ARE NORMALISED

$\rightarrow \max | \phi_i^{(j)} | = 1$
 $\rightarrow \text{NORM } \underline{\phi}_i = 1$

THE MOTION OF THE SYSTEM MODELLED IN THE FIRST SLIDE CAN BE COMPLETELY REPRESENTED BY:

$$\underline{u}(t) = \sum_{i=1}^N \underline{\phi}^{(i)} \left(A_i \cos \omega_i t + B_i \sin \omega_i t \right)$$

unknowns $\rightarrow 2N$! FROM INITIAL CONDITIONS!

MODAL ANALYSIS: STUDY OF THE SYSTEM THROUGH THE PROPERTIES OF VIBRATION MODES

MODAL MATRIX

$$\underline{\Phi} = \begin{bmatrix} \phi_1^{(1)} & \vdots & \vdots & \vdots \\ \vdots & \ddots & \ddots & \ddots \\ \phi_N^{(1)} & \vdots & \vdots & \vdots \end{bmatrix} \begin{matrix} \phi_1^{(N)} \\ \vdots \\ \phi_N^{(N)} \end{matrix} \quad (N \times N)$$

$\underline{\phi}^{(1)}$ $\underline{\phi}^{(N)}$

SPECTRAL MATRIX

$$\underline{\Omega} = \begin{bmatrix} \omega_1^2 & & & 0 \\ & \ddots & & \\ & & \ddots & \\ 0 & & & \omega_N^2 \end{bmatrix} \quad (N \times N)$$

INTRODUCE THE PRINCIPAL COORDINATES $(\underline{z}(t))$ = VECTOR OF N COMPONENTS

$$\underline{u}(t) = \underline{\Phi} \underline{z}(t), \quad \dot{\underline{u}}(t) = \underline{\Phi} \dot{\underline{z}}(t), \quad \ddot{\underline{u}}(t) = \underline{\Phi} \ddot{\underline{z}}(t)$$

(1) \rightarrow $\underline{M} \underline{\Phi} \ddot{\underline{z}}(t) + \underline{K} \underline{\Phi} \underline{z}(t) = \underline{0} \Rightarrow$

$$\underline{\Phi}^T \underline{M} \underline{\Phi} \ddot{\underline{z}}(t) + \underline{\Phi}^T \underline{K} \underline{\Phi} \underline{z}(t) = \underline{0}$$

$$\underline{\hat{M}}$$

$$\underline{\hat{K}}$$

DIAGONAL MATRICES

$$\underline{\hat{M}} = \begin{bmatrix} \hat{m}_1 & & & 0 \\ & \hat{m}_2 & & \\ & & \ddots & \\ 0 & & & \hat{m}_n \end{bmatrix}; \quad \underline{\hat{K}} = \begin{bmatrix} \hat{k}_1 & & & 0 \\ & \hat{k}_2 & & \\ & & \ddots & \\ 0 & & & \hat{k}_n \end{bmatrix}$$

$$\begin{cases} \hat{m}_1 \ddot{z}_1 + \hat{k}_1 z_1 = 0 \\ \hat{m}_2 \ddot{z}_2 + \hat{k}_2 z_2 = 0 \\ \vdots \\ \hat{m}_n \ddot{z}_n + \hat{k}_n z_n = 0 \end{cases}$$

N
INDEPENDENT
RESONATORS

$$\hat{m}_i = \underline{\phi}^{(i)} \cdot \underline{M} \underline{\phi}^{(i)}$$

$$\hat{k}_i = \underline{\phi}^{(i)} \cdot \underline{K} \underline{\phi}^{(i)}$$

let us study the 1st Eq:

$$\hat{m}_1 \ddot{z}_1(t) + \hat{K}_1 z_1(t) = 0 \quad \rightarrow \quad \ddot{z}_1(t) + \omega_1^2 z_1(t) = 0$$

$$\omega_1^2 = \frac{\hat{K}_1}{\hat{m}_1}$$